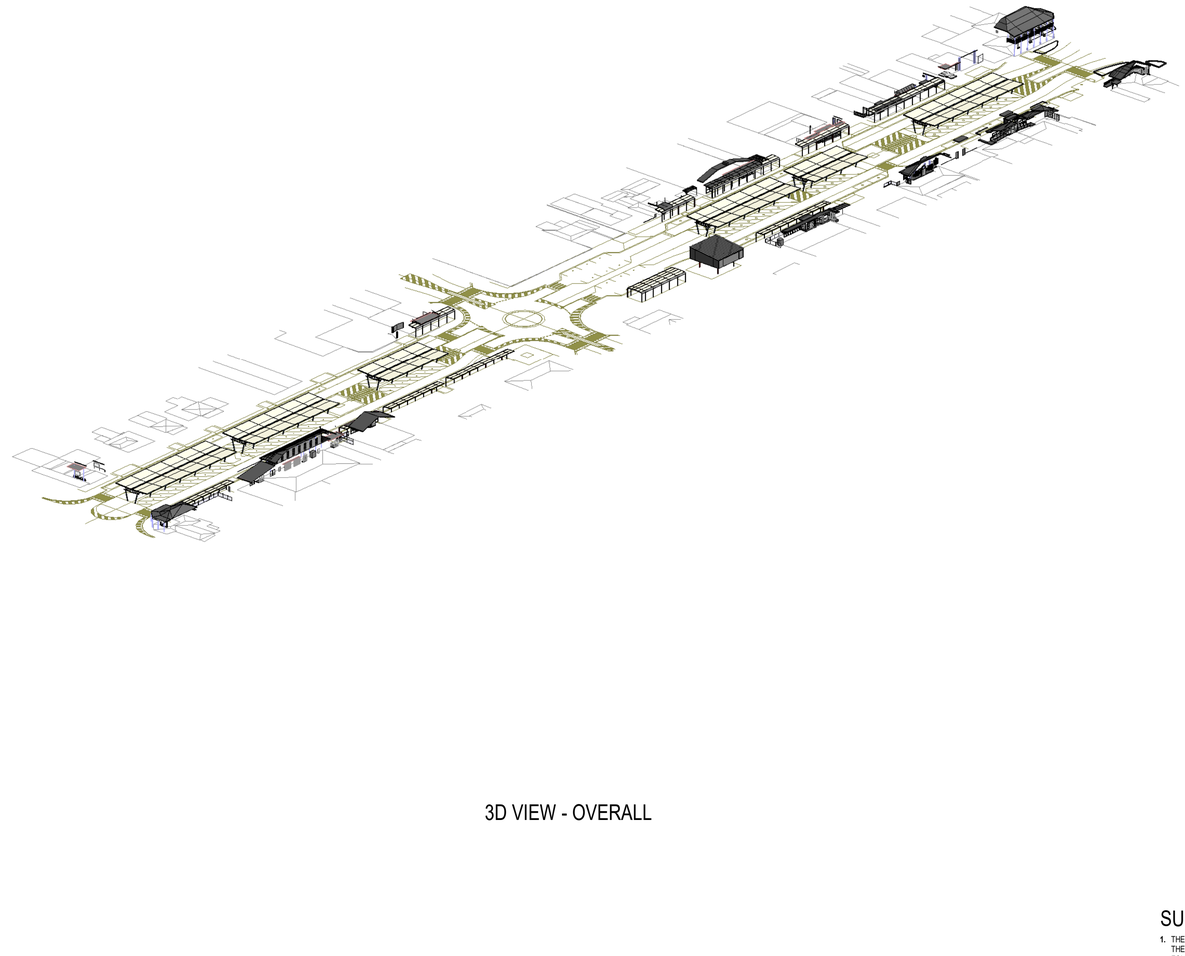
SCARR STREET

DRAWING LIST

DRAWING NUMBER	DRAWING NUMBER	DRAWING NAME	CURRENT REVISION	DATE	DESCRIPTION
S00-001	00-001	COVER PAGE AND DRAWING LIST	B	24/05/2024	FOR INFORMATION
S00-011	00-011	GENERAL NOTES PAGE - SHEET 1	B	24/05/2024	FOR INFORMATION
S00-012	00-012	GENERAL NOTES PAGE - SHEET 2	B	24/05/2024	FOR INFORMATION
S00-013	00-013	GENERAL NOTES PAGE - SHEET 3	B	24/05/2024	FOR INFORMATION
S00-111	00-111	SAFETY IN DESIGN NOTES	B	24/05/2024	FOR INFORMATION
S01-001	01-001	OVERALL SITE LAYOUT AND MARKING PLAN	B	24/05/2024	FOR INFORMATION
S01-100	01-100	ZONE 1 DEMOLITION	B	24/05/2024	FOR INFORMATION
S01-101	01-101	ZONE 1 LAYOUT	B	24/05/2024	FOR INFORMATION
S01-200	01-200	ZONE 2 DEMOLITION	B	24/05/2024	FOR INFORMATION
S01-201	01-201	ZONE 2 LAYOUT	B	24/05/2024	FOR INFORMATION
S01-300	01-300	ZONE 3 DEMOLITION	B	24/05/2024	FOR INFORMATION
S01-301	01-301	ZONE 3 LAYOUT	B	24/05/2024	FOR INFORMATION
S01-401	01-401	ZONE 4 LAYOUT	B	24/05/2024	FOR INFORMATION
S01-500	01-500	ZONE 5 DEMOLITION	B	24/05/2024	FOR INFORMATION
S01-501	01-501	ZONE 5 LAYOUT	B	24/05/2024	FOR INFORMATION
S01-600	01-600	ZONE 6 DEMOLITION	B	24/05/2024	FOR INFORMATION
S01-601	01-601	ZONE 6 LAYOUT	B	24/05/2024	FOR INFORMATION
S01-701	01-701	ZONE 7 LAYOUT	B	24/05/2024	FOR INFORMATION



3D VIEW - OVERALL

SURVEY AND SITE SETOUT:

1. THE COORDINATE SYSTEM USED IN THE REVIT MODEL & SHOWN ON THE DRAWINGS IS BASED ON THE INFORMATION FROM THE FOLLOWING SURVEY INFORMATION:

   SURVEYOR: xxxx
   PROJECT #: xxxx
   DATE: xxxx
   RP DESC: xxxx
   LEV: xxxx
   PSM #: xxxx
   PSM LOCAL COORDS: **E 00000.000 N 00000.000 RL. 00.00m**

   TO BE CONFIRMED UPON RECEIPT OF PROJECT SURVEY

2. THE LOCATION AND SETOUT OF ANY EXISTING STRUCTURE AND SERVICES ARE TO BE CONFIRMED ON SITE PRIOR TO ANY WORKS COMMENCING.

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

Client

Address

ptg consulting

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North

N

Project

SCARR STREET

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MILTON, QLD
4064

Drawing Title

COVER PAGE AND DRAWING LIST

Drawn	Approved	Scale @ A1	Drawing Status
SC		1 : 1	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S00-001	B

C:\Onshape\Protest Engineering\PTG 00093 - Scarr Street Revitalisation Project, Cloncurry - General\4-Structural\1-Dgs\7-RVT\PTG_00093 - Scarr Street Revitalisation Project.rvt

24/05/2024 12:05:52 PM

SCARR STREET

GENERAL:

1. THESE DRAWINGS SHALL BE READ IN CONJUNCTION WITH ALL ARCHITECTURAL AND OTHER CONSULTANTS' DRAWINGS AND SPECIFICATIONS AND WITH SUCH OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED DURING THE COURSE OF THE CONTRACT. ANY DISCREPANCY SHALL BE REFERRED TO THE STRUCTURAL ENGINEERS BEFORE PROCEEDING WITH THE WORK.

2. SETTING-OUT DIMENSIONS AND SIZES OF STRUCTURAL MEMBERS SHALL NOT BE OBTAINED BY SCALING THE STRUCTURAL DRAWINGS. ANY SETTING-OUT DIMENSIONS SHOWN IN THE STRUCTURAL DRAWINGS SHALL BE CHECKED BY THE CONTRACTOR BEFORE CONSTRUCTION COMMENCES.

3. UNLESS NOTED OTHERWISE ALL LEVELS ARE IN METRES AND ALL DIMENSIONS ARE IN MILLIMETRES.

4. CONTRACTOR TO COORDINATE ALL SERVICES TO AVOID CLASHES WITH STRUCTURAL ELEMENTS. ALL EXISTING SERVICES SHALL BE LOCATED PRIOR TO THE COMMENCEMENT OF WORKS.

5. ALL MATERIALS AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE RELEVANT AND CURRENT AUSTRALIAN STANDARDS AND WITH THE BY-LAWS AND ORDINANCES OF THE RELEVANT BUILDING AUTHORITIES EXCEPT WHERE VARIED BY THE PROJECT SPECIFICATION OR WRITTEN INSTRUCTION.

6. CONTRACTOR TO PROVIDE A MINIMUM OF 48 HOURS NOTICE FOR ALL ENGINEERING INSPECTIONS.

7. DURING CONSTRUCTION, THE STRUCTURE, NEIGHBOURING STRUCTURES AND ADJACENT SERVICES SHALL BE MAINTAINED IN A SAFE AND STABLE CONDITION. NO PART SHALL BE OVERSTRESSED. TEMPORARY SUPPORT AND BRACING SHALL BE PROVIDED BY THE CONTRACTOR AS REQUIRED TO KEEP THE WORKS AND EXCAVATIONS STABLE AT ALL TIMES.

8. CONTRACTOR TO OBTAIN WRITTEN INSTRUCTION FOR VARIATIONS, ALTERNATIVE DETAILS OR WHERE DETAILS HAVE NOT BEEN INCLUDED WITHIN THE CURRENT DOCUMENTATION PRIOR TO PROCEEDING WITH WORKS.

9. NO PENETRATIONS, CORING OR CHASING OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE PERMITTED WITHIN STRUCTURAL ELEMENTS WITHOUT WRITTEN APPROVAL.

10. PROPRIETARY ITEMS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS SPECIFICATIONS. ELEMENTS DESIGNED BY OTHERS SHALL BE INSPECTED AND CERTIFIED BY THE SAME AND SHALL NOT BE INCLUDED WITHIN THE STRUCTURAL CERTIFICATION PROVIDED BY THE STRUCTURAL ENGINEERS.

DESIGN CRITERIA:

1. **EXPOSURE CLASSIFICATION (REFER AS 3600):**
   EXTERNAL: **B1** INTERNAL: **A1** FOOTINGS: **A2**

2. **FIRE RESISTANCE RATING:**
   CLASS OF BUILDING: **7a,9,10**
   TYPE OF CONSTRUCTION: **C**

3. **WIND LOADING:**
   REGION: **A0**
   TERRAIN CATEGORY: **TC2**
   REFER TO **AS 1170.2** FOR ALL WIND VELOCITIES & PRESSURE COEFFICIENTS. ANY SPECIAL REQUIREMENTS ARE LISTED BELOW :
   -

4. **EARTHQUAKE LOADING (REFER AS 1170.4):**
   IMPORTANCE LEVEL: **2**
   SITE SUBSOIL CLASS:
   HAZARD FACTOR: **0.08**
   EARTHQUAKE DESIGN CATEGORY: **I**
   STRUCTURAL DUCTILITY FACTOR: **μ = 3**

5. **BUILDING DESIGN LOADS:**
   THE STRUCTURAL COMPONENTS DETAILED ON THESE DRAWINGS HAVE BEEN DESIGNED IN ACCORDANCE WITH **AS/NZS 1170.1** AND LOCAL AUTHORITY ORDINANCES FOR THE FOLLOWING LOADINGS.

ELEMENT	**IMPOSED LOADS (kPa)**	
AWNING ROOFS	**G = 0.4**	**Q = 0.25**

6. **DESIGN LIFE FOR DURABILITY:**
ALL ELEMENTS THAT HAVE BEEN SPECIFIED AS "DESIGN AND CONSTRUCT" SHALL BE DESIGNED FOR DURABILITY USING A DESIGN LIFE OF **50 YEARS**. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION THAT DEMONSTRATES COMPLIANCE.

GEOTECHNICAL INFORMATION:

1. ALL EARTHWORK, FOUNDATION AND SLAB ON GROUND INFORMATION CONTAINED WITHIN THESE DRAWINGS IS PROVISIONAL AND BASED ON THE INFORMATION CONTAINED IN THE GEOTECHNICAL SITE INVESTIGATION REPORT:

   No..... **XXXXX**
   PRODUCED BY:..... **XXXXX**
   DATED:..... **XXXXX**

2. THE SITE HAS BEEN CLASSIFIED IN ACCORDANCE WITH **AS 2870** - RESIDENTIAL SLABS AND FOOTINGS CONSTRUCTION AS FOLLOWS:

   CLASS P (SOIL REACTIVITY: H1)

SUBGRADE PREPARATION:

1. THE SITE SHALL BE STRIPPED AND EXCAVATED TO THE LEVELS SHOWN ON THE RELEVANT DRAWINGS. APPROVED PREPARED SUB BASE TO HAVE MINIMUM BEARING CAPACITY OF: **2500kPa**

2. **ALL FILL** SHALL BE PLACED IN ACCORDANCE WITH **SECTION 6** OF **AS 3798** AND TESTED IN ACCORDANCE WITH **SECTION 7** OF **AS 3798**. BACKFILL AND FILL SHALL BE COMPACTED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT IN **250mm** MAXIMUM DEPTH LOOSE LAYERS.

SUBMISSIONS:

1. WHERE RELEVANT, PTG CONSULTING WILL REQUEST DOCUMENTATION VERIFYING THE PERFORMANCE OF STRUCTURAL ELEMENTS IN ORDER TO PROVIDE THE STRUCTURAL FORM 12. THIS MAY INCLUDE (BUT NOT LIMITED TO) THE FOLLOWING:

   FOUNDATIONS:

   a. GEOTECHNICAL REPORT CERTIFYING THAT FOUNDATIONS HAVE ACHIEVED ADEQUATE REQUIRED PERFORMANCE.

   PILING:

   a. FORM 12 AND FORM 15 FOR ALL DESIGN AND CONSTRUCT PILED ELEMENTS.

   CONCRETE:

   a. CONCRETE TEST RESULTS.
   b. REINFORCEMENT THIRD PARTY PROCESSOR CERTIFICATION.

   POST INSTALLED ANCHORS:

   a. ANCHOR TEST RESULTS.

   STEEL:

   a. MILL TEST RESULTS.

   MASONRY:

   a. GROUT TEST RESULTS.
   b. REINFORCEMENT THIRD PARTY PROCESSOR CERTIFICATION.
   c. MASONRY UNIT COMPRESSIVE STRENGHT TEST RESULTS.

GENERAL PILING NOTES:

1. ALL PILING IS TO BE IN ACCORDANCE WITH AS 2159.

2. PILES SHALL BE PROVIDED BY THE CONTRACTOR ON A DESIGN AND CONSTRUCT BASIS (UNLESS DESIGNED AND DETAILED ON THE PROJECT DRAWINGS) TO RESIST THE ALLOWABLE WORKING LOADS (INCLUDING TENSION LOADS TRANSMITTED FROM THE STRUCTURE OVER) SHOWN ON THE PROJECT DOCUMENTATION.

3. **GEOTECHNICAL INFORMATION:** REFER TO THE GEOTECHNICAL INFORMATION NOTE FOR SITE INVESTIGATION INFORMATION. AT THE TIME OF TENDER, THE CONTRACTOR IS TO ADVISE IF SUPPLIED INVESTIGATION IS INADEQUATE AND IF ADDITIONAL INVESTIGATIONS ARE REQUIRED.

4. ALL PILE DESIGN DETAILS, ALONG WITH RPEQ CERTIFICATION (FORM 15) OF THE PILE DESIGN, SHALL BE SUBMITTED TO PTG CONSULTING FOR REVIEW AND APPROVAL PRIOR TO COMMENCEMENT OF CONSTRUCTION. THE SUBMITTED PILE DRAWINGS SHALL INCLUDE THE FOLLOWING:
   - DETAILS OF CONNECTION BETWEEN PILES AND THE STRUCTURE OVER.
   - IDENTIFICATION OF EACH PILE TYPE AND CAPACITY.
   - INDICATIVE PILE LENGTH.
   - CONFIRMATION OF SPECIFIED DURABILITY REQUIREMENTS.
   - PILE DESIGN LIFE.
   - ASSUMED GEOTECHNICAL DESIGN PARAMETERS.

   ANY REDESIGN AND REDOCUMENTATION WORKS AS A RESULT OF THE PROPOSED PILING DESIGN SHALL BE PERFORMED BY PTG CONSULTING, WITH ASSOCIATED COSTS BORNE BY THE CONTRACTOR.

5. **TOLERANCE:** ALL PILES SHALL BE INSTALLED WITH A MAXIMUM PLAN TOLERANCE OF 50mm AND A VERTICAL TOLERANCE OF 1:100. AN AS-BUILT SURVEY OF PILE LOCATIONS SHALL BE PERFORMED BY A LICENSED SURVEYOR AND SUPPLIED TO PTG CONSULTING WITHIN 14 DAYS OF COMPLETING PILING WORKS. THE SURVEY IS TO BE PROVIDED IN ELECTRONIC CAD FORMAT AND INCLUDE ALL PILE LOCATIONS, ECCENTRICITIES FROM THE NOMINATED LOCATIONS, AND CUT OFF RLS. ANY PILES THAT ARE OUTSIDE OF THE ACCEPTABLE TOLERANCE OR DIFFERENT TO THOSE SPECIFIED ON THE DRAWINGS ARE TO BE HIGHLIGHTED AND MAY REQUIRE RECTIFICATION. ALL DESIGN, DOCUMENTATION AND CONSTRUCTION COSTS ASSOCIATED WITH ANY RECTIFICATION WORKS SHALL BE BORNE BY THE CONTRACTOR. ALL REDESIGN WORKS TO PTG CONSULTING'S DOCUMENTED ELEMENTS ARE TO BE COMPLETED BY PTG CONSULTING.

6. THE PILING CONTRACTOR SHALL BE RESPONSIBLE FOR THE LOCATION AND PROTECTION OF ALL EXISTING IN-GROUND SERVICES DURING THE INSTALLATION OF PILES.

7. **ALL CAST INSITU BORED PILES** SHALL BE INSPECTED BY A PROFESSIONAL GEOTECHNICAL ENGINEER AND APPROVED PRIOR TO PLACING CONCRETE. CERTIFICATION SHALL BE PROVIDED BY AN RPEQ QUALIFIED GEOTECHNICAL ENGINEER STATING THAT THE PILES WILL PERFORM AS SPECIFIED.

8. THE PILING CONTRACTOR SHALL GUARANTEE THAT THE SETTLEMENT OF EACH PILE OR GROUP OF PILES SHALL NOT EXCEED 20mm. SUCH GUARANTEE SHALL REMAIN EFFECTIVE FOR SEVEN (7) YEARS FROM THE CERTIFIED DATE OF PRACTICAL COMPLETION OF THE BUILDING.

9. **SUBSTITUTIONS:** WHERE ALTERNATIVE DESIGNS ARE PROPOSED TO THE DOCUMENTED PILING SOLUTION, SUBMIT COMPLETE DETAILS OF THE PROPOSED SYSTEM INCLUDING ALL DIMENSIONS, SIZES, CAPACITIES, SETTLEMENTS, AND METHODS OF INSTALLATION. THE CONTRACTOR SHALL BEAR THE COST OF ANY PILE CAP REDESIGN AND REDOCUMENTATION THAT MAY BE REQUIRED.

10. **INSPECTION**: PROVIDE A MINIMUM OF 48 HOURS NOTICE SO THAT INSPECTION MAY BE MADE OF THE FOLLOWING:
    - INSTALLATION OF PILING (E.G. REINFORCEMENT CAGE OR LINERS IN PLACE).
    - PILE LOAD TESTS.

11. **REINFORCEMENT:** PROVIDE SPACERS ON THE REINFORCEMENT CAGE TO MAINTAIN THE REQUIRED COVER NOMINATED ON PILE DRAWINGS. DURING INSTALLATION OF REINFORCEMENT IN UNCASED HOLES KEEP THE REINFORCEMENT CAGE CLEAR OF THE SIDES OF THE HOLE. REINFORCEMENT IS TO EXTEND ABOVE THE PILE CUT-OFF LEVEL AND IS NOT TO BE BENT OVER.

12. **CONCRETE PLACEMENT** SHALL COMPLY WITH THE REQUIREMENTS OF AS 2159. CONCRETE SHALL BE PLACED TO FILL THE ENTIRE VOLUME OF THE PILE WITHOUT THE FORMATION OF VOIDS CAUSED BY ENTRAPPED AIR, LACK OF COMPACTION OR SEGREGATION. CONCRETE SHALL BE PLACED IN SUCH A MANNER THAT THE POSITION OF THE REINFORCEMENT IS MAINTAINED. THE VOLUME OF PLACED CONCRETE SHALL BE RECORDED.

13. **DAMAGE:** THE CONTRACTOR IS TO PERFORM PILING OPERATIONS IN SUCH A MANNER SO AS NOT TO CAUSE DAMAGE OR DISTURBANCE TO ADJACENT BUILDINGS OR SERVICES. IF DAMAGE IS CAUSED TO ADJOINING PROPERTY, STOP PILING OPERATIONS AND GIVE NOTICE. ANY COSTS ASSOCIATED WITH DAMAGE CAUSED BY THE PILING OPERATIONS ARE TO BE BORNE BY THE CONTRACTOR.

14. **MONITOR:** CONTINUOUSLY MONITOR VIBRATIONS, SETTLEMENTS AND MOVEMENTS AND ENSURE THE STABILITY OF EXCAVATION AND CONSTRUCTION SLOPES IS MAINTAINED. VIBRATION TOLERANCES ARE TO BE IN ACCORDANCE WITH PTG CONSULTING'S VIBRATION LIMITS AND MONITORING NOTE. MAINTAIN PILE INTEGRITY DURING DRIVING. STOP AND RE-ASSESS HAMMER TYPE/SIZE/DROP IF DAMAGE IS DETECTED.

VIBRATION LIMITS AND MONITORING:

1. THE CONTRACTOR SHALL AT ALL TIMES COMPLY WITH THE RELEVANT STATE AND/OR LOCAL GOVERNMENT NOISE PROVISIONS **(QLD PROJECTS: "ENVIRONMENTAL PROTECTION (NOISE) POLICY 2019")** AND THE CONDITIONS OF THE APPROVED DEVELOPMENT APPLICATION.

2. **STRUCTURAL BORNE VIBRATIONS** SHALL BE LIMITED TO MINIMISE THE RISK OF DAMAGE TO EXISTING STRUCTURES AND INFRASTRUCTURE.

3. THE CONTRACTOR SHALL ENGAGE A VIBRATION SPECIALIST TO CONFIRM VIBRATION LIMITS FOR EXISTING STRUCTURES AND INFRASTRUCTURE.

4. **WHERE VIBRATION CONTROL IS REQUIRED,** THE CONTRACTOR SHALL, AT THE TIME OF TENDER, PROVIDE DETAILS OR TECHNIQUES FOR THE REDUCTION OF EXCESSIVE VIBRATION (E.G. PREBORING ETC.) AND/OR NOISE.

5. UNLESS FORMALLY VERIFIED BY A VIBRATION SPECIALIST, THE FOLLOWING VIBRATION LIMITS SHALL APPLY:

   5.1.**VIBRATIONS TO NON HERITAGE STRUCTURES** SHALL NOT EXCEED THE PEAK PARTICLE VOLOCITY (PPV) LIMITS GIVEN IN DIN 4150 AS NOTED BELOW:

TYPE OF STRUCTURE	SHORT TERM / TRANSIENT			
	FREQUENCY AT FOUNDATION			UPPERMOST FLOOR
	0-10 Hz	10-50 Hz	50-100Hz	ALL FREQUENCIES
COMMERCIAL/ INDUSTRIAL	20	20-40	40-50	40
RESIDENTIAL	5	5-15	15-20	15

TYPE OF STRUCTURE	LONG TERM / RESONANT
	UPPERMOST FLOOR
	ALL FREQUENCIES
COMMERCIAL/ INDUSTRIAL	10
RESIDENTIAL	5

   5.2. **SENSITVIE/HISTORIC/HERITAGE STRUCTURES:** ADVICE FROM VIBRATION SPECIALIST TO BE OBTAINED

   5.3. **IN GROUND SERVICES** ARE NOT TO BE DAMAGED BY CONSTRUCTION OPERATIONS. ADVICE ON VIBRATION LIMITS IS TO BE OBTAINED FROM A VIBRATION SPECIALIST WHERE REQUIRED.

6. **MONITOR.** A VIBRATION MONITORING PLAN SHALL BE DEVELOPED FOR THE WORKS BY THE CONTRACTOR IN CONJUCTION WITH THE RESPECTIVE CONSULTANTS, AND SHOULD ADDRESS THE FOLLOWING:

   a) PROVIDE AND MAINTAIN VIBRATION MONITORING EQUIPMENT FOR THE PERIOD OF THE CONTRACT, TO MEASURE VIBRATIONS AT AFFECTED BUILDINGS AND MAKE AVAILABLE ALL RECORDS.

   b) INSTALL VISIBLE AND AUDIBLE ALARMS TO INSTANTLY INDICATE EXCEEDENCE.

   c) TRAIN ALL MACHINE OPERATORS IN VIBRATION CONTROLLED TECHNIQUES BEFORE ON-SITE ENGAGEMENT.

7. **DAMAGE.** IF DAMAGE IS CAUSED TO ADJOINING PROPERTY, STOP VIBRATION-CAUSING OPERATIONS AND PROVIDE NOTICE. ANY COSTS ASSOCIATED WITH DAMAGE CAUSED BY THE VIBRATION-CAUSING OPERATIONS SHALL BE BORNE BY THE CONTRACTOR.

8. **TESTING.** THE CONTRACTOR IS TO ALLOW FOR SUFFICIENT TESTING AND INSPECTIONS TO CONFIRM THE PARAMATERS USED IN THE DESIGN, IN ACCORDANCE WITH AS2159. CONCRETE IS TO BE TESTED IN ACCORDANCE WITH THE CONCRETE NOTE.

GENERAL FOOTINGS:

1. THIS FOOTING DESIGN HAS BEEN PREPARED BASED ON INFORMATION PROVIDED IN THE GEOTECHNICAL REPORT. IT IS THE BUILDERS' RESPONSIBILITY TO MAKE REFERENCE TO THIS REPORT AND ITS RECOMMENDATIONS.

2. THE BUILDER MUST BE AWARE THAT FOUNDATIONS MAY VARY BETWEEN TEST LOCATIONS REFERRED TO IN THE GEOTECHNICAL REPORT. IF FOUNDATION CONDITIONS DIFFER FROM THOSE DESCRIBED IN THE GEOTECHNICAL REPORT IN PERFORMING EARTHWORKS OR FOOTING EXCAVATION, THEN THE STRUCTURAL ENGINEERS MUST BE NOTIFIED IN WRITING IMMEDIATELY AS AN AMENDMENT TO THE SITE CLASSIFICATION AND/OR DESIGN MAY BE REQUIRED.

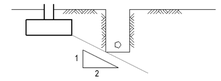
3. THIS DESIGN HAS BEEN BASED ON THE ASSUMPTION THAT THE FOUNDING MATERIAL SATISFIES THE MINIMUM BEARING CAPACITY NOMINATED ON THE DRAWINGS. FURTHERMORE, THE PROPOSED LOCATION OF THE STRUCTURE IS NOT SUBJECT TO POSSIBLE GEOTECHNICAL OR OTHER SLOPE INSTABILITY PROBLEMS.

4. FOOTINGS SHALL BE LOCATED CENTRALLY UNDER WALLS AND COLUMNS UNLESS NOTED OTHERWISE.

5. ALL LOOSE MATERIALS AND WATER TO BE CLEANED OUT OF THE FOUNDATION. FORM WORK TO BE USED WHERE THE SIDES OF THE FOUNDATION ARE NOT STABLE.

6. A 50mm MINIMUM BLINDING LAYER SHOULD BE APPLIED TO THE BASE OF ALL FOUNDATIONS IMMEDIATELY AFTER VERIFICATION OF THE BEARING CAPACITY BY THE GEOTECHNICAL ENGINEER. WHERE THE FOUNDING MATERIAL IS DEEPER THAN REQUIRED FOR THE FOOTING, THE EXCAVATION IS TO BE BACKFILLED WITH A WEAK MIX CONCRETE (N10) TO THE UNDERSIDE OF THE FOOTING.

7. UNLESS SPECIFICALLY DETAILED ON DRAWINGS, THE LIMITATIONS OF EXCAVATIONS NEAR FOOTINGS SHALL BE AS FOLLOWS:



8. EXCAVATION NEAR EXISTING FOOTINGS SHALL NOT EXTEND BELOW FOUNDATION LEVEL WITHOUT THE APPROVAL OF THE STRUCTURAL ENGINEER.

9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ANY EXCAVATION IN A STABLE CONDITION WITHOUT ADVERSELY AFFECTING SURROUNDING PROPERTY, INCLUDING SERVICES. THIS INCLUDES OBTAINING ALL NECESSARY APPROVALS FOR SHORING AND ANCHORING SYSTEMS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING AND LOCATING ALL EXISTING UNDERGROUND SERVICES PRIOR TO ANY EXCAVATION COMMENCING.

10. DEPTHS SPECIFIED FOR FOOTINGS ARE MINIMUM DIMENSIONS ONLY. GREATER DEPTH MAY BE NECESSARY TO ACHIEVE SPECIFIED FOUNDING BASE.

11. IF THE MATERIAL UNDERLYING THE SITE IS EXPANSIVE CLAY, PRECAUTIONS ARE TO BE TAKEN TO AVOID MOISTURE VARIATIONS. IF THE MATERIAL UNDERLYING THE SITE IS EXPANSIVE CLAY, THE FOLLOWING PRECAUTIONS ARE TO BE TAKEN TO AVOID MOISTURE VARIATIONS.
    A. AVOID LANDSCAPE PLANTING CLOSE TO FOOTINGS.
    B. PROMPT REPAIR OF LEAKING SERVICES.
    C. FINISHED LEVELS SHALL ENSURE SURFACE WATER CAN NOT POND AGAINST FOOTING.

GENERAL MASONRY:

1. ALL MASONRY SHALL BE IN ACCORDANCE WITH THE MASONRY CODE **AS 3700**.

2. BLOCKS SHALL HAVE A MINIMUM UNCONFINED COMPRESSIVE STRENGTH OF **15 MPa** TO **AS 4455** LAID IN STRETCHER BOND. MASONRY UNITS FOR REINFORCED BLOCKWORK SHALL BE 'H' BLOCK, DOUBLE 'U' OR 'A' BLOCK CONFIGURATION. LINTEL BLOCKS SHALL BE USED FOR ALL LINTELS

3. BRICKS SHALL HAVE A MINIMUM UNCONFINED COMPRESSIVE STRENGTH OF **30 MPa** TO **AS 4455** LAID IN STRETCHER BOND.

4. GROUT SHALL BE GRADE **S20** WITH **10mm** MAXIMUM AGGREGATE SIZE AND 200 mm MAXIMUM SLUMP.

5. WALL TIES IN CAVITY WALLS ARE TO HAVE A DURABILITY CLASSIFICATION **R3**, TO BE MEDIUM DUTY TO **AS 2699** AND ARE TO BE AT 600mm MAX CENTRES BOTH WAYS GENERALLY, AND AT 300mm MAX. CENTRES AROUND OPENINGS AND JOINTS UNLESS DETAILED OTHERWISE.

6. **MORTAR** SHALL BE **1:1:6 CEMENT:LIME:SAND**. DURABILITY CLASSIFICATION **M3**. WATER RETENTIVITY OF MORTAR SHALL MATCH THE INITIAL RATE OF ABSORBTION OF MASONRY UNITS. JOINTS SHALL BE 10mm THICK AND TOOLED UNLESS NOTED OTHERWISE.

7. **CLEANOUT OPENINGS** SHALL BE FORMED AT THE BASE OF ALL POURS. ALL MORTAR DROPPINGS AND OTHER OBSTRUCTIONS SHALL BE REMOVED FROM BLOCK CAVITY. **MORTAR "FINS"** PROTRUDING MORE THAN 10mm FROM JOINTS INSIDE CORES SHALL BE DISLODGED VIA RODDING AND REMOVED THROUGH CLEANOUT OPENINGS PRIOR TO POURING GROUT.

8. **CONTROL JOINTS** IN BLOCK MASONRY WALLS ARE TO BE LOCATED IN ACCORDANCE WITH **PTG CONSULTING** DOCUMENTATION, UNLESS APPROVAL IS GRANTED FOR ALTERNATIVE LOCATIONS. FOR MASONRY VENEERS ONLY, REFER ARCHITECTURAL DRAWINGS FOR LOCATIONS OF CONTROL JOINTS.

9. **MAXIMUM PERMITTED FILL HEIGHT** OF BLOCKWORK IS 3.0m FOR 190 AND 290 BLOCKS AND 1.4m FOR 140 BLOCKS. BLOCKWORK SHALL NOT BE FILLED TO A HEIGHT GREATER THAN 1200mm WITHOUT SUITABLE SHORING.

10. A 300mm MINIMUM WIDE STRIP OF COARSE GRAINED MATERIAL IS TO BE PLACED BEHIND ALL RETAINING WALLS TOGETHER WITH AG DRAINS CONNECTED TO SITE DRAINAGE SYSTEM.

11. **WATERPROOFING** SHALL BE IN ACCORDANCE WITH ARCHITECT'S REQUIREMENTS.

12. **PENETRATIONS: PTG CONSULTING** SHALL BE NOTIFIED OF ANY PENETRATION LARGER THAN 200 x 200mm NOT DETAILED ON THE DRAWINGS. PENETRATIONS SHALL NOT BE LOCATED IN REINFORCED CORES AND ARE NOT PERMITTED IN LOAD-BEARING WALLS LESS THAN 1000mm IN LENGTH WITHOUT PRIOR APPROVAL FROM **PTG CONSULTING**.

13. **ALL REINFORCING STEEL** (INCLUDING BAR LAPS) SHALL BE IN ACCORDANCE WITH THE REINFORCING STEEL NOTE.

14. **WET-SETTING** OF STARTER BARS IS NOT PERMITTED UNDER ANY CIRCUMSTANCE. MASONRY WALL STARTER BARS ARE TO BE TIED TO THE FOUNDATION REINFORCEMENT.

15. **INSPECTION:** PROVIDE A MINIMUM OF 48 HOURS NOTICE FOR INSPECTIONS. ALL REINFORCEMENT SHALL BE ACCURATELY POSITIONED, ADEQUATELY SUPPORTED, AND THEN INSPECTED BY **PTG CONSULTING** BEFORE ANY GROUT IS PLACED. IF THE LINTEL HAS GREATER THAN 2 COURSES OF BLOCK OVER, A PHOTO DEMONSTRATION OF THE LINTEL REINFORCEMENT IS REQUIRED.

16. **GROUT** SHALL BE TESTED IN ACCORDANCE WITH AS 1379 AND THE SAMPLING TABLE BELOW. TEST RESULTS SHALL BE SUBMITTED TO PTG CONSULTING. FOR EACH STRENGTH GRADE THE FREQUENCY OF TESTING SHOULD BE AS FOLLOWS:

ELEMENT BEING POURED	**FREQUENCY OF TESTING PER POUR**
COREFILLING	ONE SAMPLE PER 50m³ OR PART THERE OF FOR EACH PROPERTY OF GROUT

UPON REQUEST, THE CONTRACTOR SHALL SUBMIT TEST RESULTS FOR COMPRESSIVE STRENGTH OF MASONRY UNITS IN ACCORDANCE WITH AS4456.4

17. **TEMPORARY SUPPORT**: IF THE FINAL STABILITY OF THE MASORNY IS DEPENDENT ON CONSTRUCTION OF (STRUCTURAL) ELEMENTS AFTER THE BRICKWORK AND BLOCKWORK IS COMPLETED, PROVIDE ADEQUATE TEMPORARY SUPPORT OR BRACING.

18. **GROUTING:** DO NOT COMMENCE UNTIL GROUT SPACES HAVE BEEN CLEANED OUT AND THE MORTAR JOINTS HAVE ATTAINED SUFFICIENT STRENGTH TO RESIST BLOW-OUTS. COMPACT BY VIBRATION OR BY RODDING TO ACHIEVE ADEQUATE COMPACTION AND REMOVE VOIDS.

GENERAL MASONRY CONT'D:

19. **BRICK LINTELS**: PROVIDE LINTELS FOR BRICK WALLS IN ACCORDANCE WITH THE LINTEL SCHEDULE. PACK MORTAR BETWEEN ANY VERTICAL COMPONENT AND SUPPORTED MASONRY UNITS. FOR UNEQUAL ANGLES, INSTALL THE LONG LEG VERTICAL. MINIMUM BEARING EACH END:
    - SPAN ≤ 1000mm: 100mm
    - SPAN > 1000mm ≤ 3000mm: 150mm.
    - SPAN > 3000mm: TO STRUCTURAL DRAWINGS.

    PROVIDE TEMPORARY PROPS TO LINTELS TO PREVENT DEFLECTION OR ROTATION.
    - MINIMUM PROPPING PERIOD: 7 DAYS

BRICK LINTEL SCHEDULE

	CONSTRUCTION TYPE			
STEEL SECTION	**A**	**B**	**C**	**D**
ANGLES	MAX. CLEAR LINTEL SPAN (mm)			
90 x 90 x 6 EA	3010	2050	2050	1570
90 x 90 x 8 EA	3010	2170	2170	1810
100 x 100 x 6 EA	3130	2290	2290	1810
100 x 100 x 8 EA	3370	2410	2410	1930
150 x 90 x 8 UA	4210	3370	3370	2770
150 x 100 x 10 UA	4330	3490	3610	3010
FLATS				
75 x 8	490	250	-	-
75 x 10	610	250	250	250

LEGEND:
CONSTRUCTION TYPES
A - SUPPORTING THE EXTERNAL LEAF OF BRICK VENEER
B - SUPPORTING EXTERNAL OR INTERNAL LEAF OF CAVITY BRICKWORK AND TILED ROOF.
C - SUPPORTING A SINGLE LEAF WALL AND SHEET METAL ROOF.
D - SUPPORTING A SINGLE LEAF WALL AND TILED ROOF.

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

Client

Address

ptg consulting
a: Level 3, 159 Coronation Drive (CNR Cribb St), Milton, QLD, 4064
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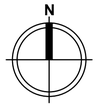
North



Project
SCARR STREET
Address
LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title
GENERAL NOTES PAGE - SHEET 1

Drawn	Approved	Scale @ A1	Drawing Status
SC		1 : 1	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S00-011	B

C:\OneDrive\Protest Engineering\PTG 00093 - Scarr Street Revitalisation Project. Cloncurry - General\A4-Structural\1-Drgs\7-RVT\PTG_00093 - Scarr Street Revitalisation Project.rvt

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SCARR STREET

GENERAL CONCRETE:

1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH **AS 3600**.

2. REFER REINFORCING STEEL NOTE FOR REINFORCING REQUIREMENTS.

3. **OFF FORM SURFACE FINISH**. FORMWORK IS TO BE IN ACCORDANCE WITH THE FORMWORK CODE **AS 3610**. REFER ARCHITECT FOR CONCRETE FINISH SPECIFICATION. IF NO ARCHITECTURAL SPECIFICATION IS PROVIDED, CONCRETE EXPOSED TO VIEW IN THE FINAL PROJECT IS TO BE CLASS 2 FINISH AND CLASS 4 ELSEWHERE.

4. **UNFORMED SURFACE FINISH**, REFER ARCHITECT.

5. NO ADMIXTURES SHALL BE USED IN CONCRETE UNLESS APPROVED IN WRITING BY **PTG CONSULTING.**

6. CONCRETE DIMENSIONS SHOWN DO NOT INCLUDE THICKNESS OF APPLIED FINISHES. DEPTHS OF BEAMS INCLUDE SLAB THICKNESS.

7. FOR CHAMFERS, DRIP GROOVES, FILLETS ETC. REFER TO ARCHITECT'S DETAILS. MAINTAIN MINIMUM COVER TO REINFORCEMENT AT THESE LOCATIONS.

8. NO HOLES, CHASES OR EMBEDMENT OF PIPES OTHER THAN THOSE SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE MADE IN CONCRETE MEMBERS WITHOUT THE PRIOR WRITTEN APPROVAL OF **PTG CONSULTING**.

9. THE FINISHED CONCRETE SHALL BE A DENSE HOMOGENEOUS MASS, COMPLETELY FILLING THE FORMWORK, THOROUGHLY EMBEDDING THE REINFORCEMENT AND FREE OF STONE POCKETS. ALL CONCRETE, INCLUDING SLABS ON GROUND AND FOOTINGS, SHALL BE THOROUGHLY COMPACTED WITH USE OF VIBRATION. REFER **CCAA** PUBLICATION **"GUIDE TO CONCRETE CONSTRUCTION - PART V- SECTION 12 - HANDLING AND PLACING"** FOR FURTHER GUIDANCE.

10. **CONSTRUCTION JOINTS** SHALL BE FORMED AND USED ONLY WHERE SHOWN ON THE DRAWINGS OR SPECIFICALLY APPROVED BY **PTG CONSULTING.**

11. **CONSTRUCTION JOINT PREPARATION**. ROUGHEN AND CLEAN THE HARDENED CONCRETE JOINT SURFACE. REMOVE LOOSE OR SOFT MATERIAL, FREE WATER, FOREIGN MATTER AND LAITANCE. DAMPEN THE SURFACE JUST BEFORE PLACING THE FRESH CONCRETE AND COAT WITH A NEAT CEMENT SLURRY.

12. **CONCRETE SHALL BE CURED** CONTINUOUSLY FOR AT LEAST **7 DAYS**. CURING SHALL BE ACHIEVED BY THE APPLICATION OF WATER TO ACCELERATED CURING OF, OR THE RETENTION OF WATER IN, THE FRESHLY CAST CONCRETE. CURING SHALL COMMENCE AS SOON AS PRACTICABLE AFTER THE FINISHING OF ANY UNFORMED SURFACES HAS BEEN COMPLETED. WHERE RETENTION OF WATER IN THE FRESH CONCRETE RELIES ON THE APPLICATION TO EXPOSED SURFACES OF SPRAYED MEMBRANE-FORMING CURING COMPOUNDS, THE COMPOUNDS USED SHALL CONFORM WITH **AS 3799.**

13. AFTER POURING AND UNTIL FINAL TROWEL A CONTINUOUS FILM OF **ALIPHATIC ALCOHOL** SHALL BE APPLIED TO ALL EXPOSED SURFACES IN ACCORDANCE WITH THE MANUFACTURERS INSTRUCTIONS. THE FILM SHALL BE KEPT CONTINUOUS AND INTACT; RE-APPLICATION MAY BE NECESSARY IN HOT OR WINDY WEATHER OR IF FILM IS BROKEN.

14. **STRIPPING** FORMWORK TO HORIZONTAL SURFACES MAY BE STRIPPED AND RE-SHORED AFTER **7 DAYS**. VERTICAL FORMS SHALL BE STRIPPED IN ACCORDANCE WITH **AS 3610,1 APPENDIX C TABLE C2**. FOR MULTISTOREY BUILDINGS, REFER FORMWORK STRIPPING CYCLE NOTES.

15. **BACKPROPPING** SHALL REMAIN IN PLACE UNTIL CONCRETE ACHIEVES NOMINATED DESIGN STRENGTH UNLESS SPECIFICALLY APPROVED OTHERWISE. CONSTRUCTION LOADING IMPOSED BY THE TEMPORARY WORKS ENGINEER'S BACKPROPPING DESIGN SHALL NOT EXCEED THE PERMANENT IMPOSED LOADS IN ACCORDANCE WITH **AS 1170**, AND SHALL BE VALIDATED WITH RPEQ FORM 15 CERTIFICATION.

16. **INSPECTION.** PROVIDE A MINIMUM OF **48 HOURS** NOTICE SO THAT INSPECTION MAY BE MADE OF STRUCTURAL ELEMENTS. CONCRETE SHALL NOT BE PLACED UNTIL FINAL APPROVAL IS OBTAINED. PRIOR TO INSPECTION THE FOLLOWING ELEMENTS SHALL BE CORRECTLY INSTALLED AND CLEARLY VISIBLE:

    a. ANY MEMBRANES OR FILM UNDERLAY SPECIFIED TO BE INSTALLED ON THE BASE OR SUBGRADE.
    b. COMPLETED FORMWORK, CORES, FIXINGS AND EMBEDDED ITEMS FIXED IN PLACE.
    c. SURFACES OR ELEMENTS TO BE CONCEALED IN THE FINAL WORK BEFORE COVERING.
    d. ALL REINFORCEMENT(INCLUDING FASTENINGS)ACCURATELY POSITIONED AND ADEQUATELY SUPPORTED.

17. CONDUITS, PIPES, ETC, SHALL NOT BE PLACED IN COVER CONCRETE AND SHALL BE LOCATED WITHIN THE MIDDLE THIRD OF THE SLAB DEPTH. PROVIDE A MINIMUM OF 50mm CLEAR DISTANCE BETWEEN CONDUITS. CAST-IN SERVICES GREATER THAN 50mm DIAMETER NOT DOCUMENTED ON STRUCTURAL DRAWINGS SHALL NOT BE PERMITTED WITHOUT PRIOR APPROVAL FROM **PTG CONSULTING.** REFER DETAILS FOR FURTHER REQUIREMENTS.

18. BEFORE ANY CONCRETE IS PLACED (48 HOURS MINIMUM), **PTG CONSULTING** SHALL BE NOTIFIED OF ANY BEAM PENETRATION NOT DETAILED ON THE STRUCTURAL DRAWINGS, AND OF ANY SLAB PENETRATION LARGER THAN 150x150mm NOT DETAILED ON THE DRAWINGS. BARS ARE TO BE DISPLACED AROUND SLAB PENETRATIONS (MAINTAIN COVER TO PENETRATION) AND ARE NOT TO BE CUT. ANY PENETRATION NOT DOCUMENTED ON STRUCTURAL DRAWINGS AND NOT REVIEWED AND APPROVED BY **PTG CONSULTING** PRIOR TO INSPECTION MAY BE LIABLE TO REJECTION.

GENERAL CONCRETE CONT'D:

19. DO NOT BUILD NON-LOADBEARING BRICK OR BLOCKWORK ON SUSPENDED WORK UNTIL ALL SHORING HAS BEEN REMOVED U.N.O. ON STRUCTURAL DOCUMENTATION.

20. TESTING REQUIREMENTS. ALL CONCRETE SHALL BE TESTED IN ACCORDANCE WITH **AS 1012.1** AND THE SAMPLING TABLE BELOW. TEST RESULTS SHALL BE SUBMITTED TO **PTG CONSULTING.**
FOR EACH STRENGTH GRADE THE FREQUENCY OF TESTING SHOULD BE AS FOLLOWS:

ELEMENT BEING POURED (INCLUDING PRECAST ELEMENTS)	FREQUENCY OF TESTING PER POUR
PILES, FOOTINGS, SLABS, BEAMS & MASONRY CORE FILLING	ONE SAMPLE PER 50m³ OR PART THEREOF FOR EACH PROPERTY OF CONCRETE PLACED.
COLUMNS & WALLS	ONE SAMPLE PER 25m³ OR PART THEREOF FOR EACH PROPERTY OF CONCRETE PLACED.
CONTROL TESTS (i.e. EARLY STRENGTH, TENSILE STRENGTH	ONE SAMPLE PER 50m³ OR PART THEREOF FOR EACH PROPERTY OF CONCRETE PLACED, WITH A MINIMUM OF 3 TEST CYLINDERS PER SAMPLE.

WHERE SPECIFIED ON PROJECT DRAWINGS, ADDITIONAL SHRINKAGE STRAIN TEST RESULTS SHALL BE PROVIDED.

21. **MIX DESIGN**. CONCRETE IS TO BE SPECIFIED AND SUPPLIED IN ACCORDANCE WITH AS 1379. THE CONTRACTOR IS RESPONSIBLE FOR THE MIX DESIGN AND THE PRODUCTION OF THE CONCRETE IN CONFORMITY WITH THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL PROVIDE DETAILS OF THE CONCRETE MIX TO BE USED FOR APPROVAL PRIOR TO COMMENCING WORK (INCLUDING ANY ADDITION OF WATER ON SITE, CHANGES TO THE CONCRETE MIX AND TEMPERATURE CONTROL METHODS.)

22. **IF WATER IS TO BE ADDED**, COMPLY WITH **AS 1379, SECTION 4.2.3** AND THE CONCRETE SUPPLIER'S REQUIREMENTS.

23. **AMBIENT TEMPERATURE**. CONCRETE SHALL NOT BE PLACED ON SITE WHEN AMBIENT TEMPERATURES EXCEED 30°C OR FALL BELOW 10°C, UNLESS APPROVED MEASURES ARE TAKEN BY HEATING OR COOLING SO THAT THE DELIVERED CONCRETE IS WITHIN RANGE 5°C TO 35°C (OR 27°C MAX FOR CONCRETE SECTIONS EXCEEDING 600mm IN THICKNESS).

24. DRYING WINDS. ERECT BARRIERS TO PROTECT FROM DRYING WINDS (WINDS OVER 15 KNOTS AND RELATIVE HUMIDITY LESS THAN 60%) UNTIL THE CONCRETE HAS CURED. IF THE CONCRETE TEMPERATURE EXCEEDS 25°C, PROTECT THE CONCRETE USING A FOG SPRAY APPLICATION OF ALIPHATIC ALCOHOL EVAPORATION RETARDANT.

25. BASE PLATE TEMPLATES. SECURELY FASTENED TEMPLATES SHALL BE USED FOR CAST-IN BOLTS. SET OUT SHALL BE SURVEYED PRIOR TO POUR, 50mm MAX. LATERAL DISPLACEMENT OF REINFORCEMENT TO ACCOMMODATE SET OUT.

26. ENSURE THAT ELAPSED TIME BETWEEN THE WETTING OF THE MIX AND THE DISCHARGE OF THE MIX AT THE SITE IS IN CONFORMANCE WITH THE BELOW TABLE.

CONCRETE TEMPERATURE AT TIME OF DISCHARGE (C°)	MAXIMUM ELAPSED TIME (MINUTES)
10 °C - 24 °C	120min
25 °C - 27 °C	90min
28 °C - 30 °C	60min
31 °C - 35 °C	45min

FORMWORK:

1. THE DESIGN CERTIFICATION CONSTRUCTION AND PERFORMANCE OF THE FORMWORK AND FALSE WORK IS THE RESPONSIBILITY OF THE BUILDER.

2. DESIGN AND CONSTRUCTION AND STRIPPING TIMES TO COMPLY WITH **AS 3610** AND **AS 3600** UNLESS OTHERWISE APPROVED BY THE STRUCTURAL ENGINEER.
GENERALLY FORMWORK CAN BE STRIPPED AFTER **28 DAYS** OF POURING OR WHEN SLAB HAS REACHED MINIMUM DESIGN STRENGTH AS NOMINATED ON THESE DRAWINGS. THE STRUCTURAL ENGINEER IS TO BE CONTACTED FOR ALL FORMWORK STRIPPING REQUIREMENTS AND INSTRUCTIONS.

3. IT IS ANTICIPATED IN MULTI-STOREY CONSTRUCTION THAT PROPPING SHALL EXTEND AT LEAST 3 LEVELS BELOW THE FLOOR BEING CAST. PROP REMOVAL, PROGRAMMED TO AVOID DISTRESS TO PREVIOUSLY CAST FLOORS. RE-SHORING OR BACK PROPPING IS SUBJECT TO THE APPROVAL OF THE PROJECT STRUCTURAL ENGINEER.

4. DURING CONSTRUCTION, SUPPORT PROPPING IS REQUIRED WHERE LOADS FROM STACKED MATERIALS, FORMWORK AND OTHER SUPPORTED SLABS INDUCE LOADS IN A SLAB OR BEAM WHICH EXCEED THE DESIGN LOAD FOR STRENGTH OR SERVICEABILITY AT THAT AGE. ONCE THE NOMINATED 28 DAY STRENGTH HAS BEEN ATTAINED, THESE LOADS SHALL NOT EXCEED THE DESIGN SUPERIMPOSED LOADS SET OUT IN THE GENERAL NOTES.

5. THE FORMWORK SHALL NOT BE DESIGNED TO RELY ON RESTRAINT OR SUPPORT FROM THE PERMANENT STRUCTURE WITHOUT PRIOR APPROVAL FROM THE PROJECT DESIGN ENGINEER.

6. FORMWORK SHALL BE DESIGNED TO ACCOMMODATE MOVEMENTS DUE TO POST TENSIONING.

7. FOR FINISH TO FORMED CONCRETE SURFACES REFER TO SPECIFICATION AND ARCHITECTURAL DETAILS.

8. COMPOSITE STEEL BEAMS SHALL BE PROPPED PRIOR TO CASTING SLABS OVER UNLESS NOTED OTHERWISE.

REINFORCING STEEL:

1. ALL REINFORCEMENT SHALL BE IN ACCORDANCE WITH **AS/NZS 4671** AND **AS 3600**. ALL BAR CHAIRS ARE TO BE IN ACCORDANCE WITH **AS/NZS 2425.**

2. A THIRD PARTY PROCESSOR CERTIFICATION (**ACRS** OR EQUIVALENT) SHALL BE SUPPLIED WITH ALL STEEL REINFORCEMENT AT PROCUREMENT BEFORE ANY CONCRETE IS PLACED, TO GUARANTEE CONFORMANCE OF THE REINFORCEMENT TO AUSTRALIAN STANDARDS. SUBMIT PROOF OF CERTIFICATION TO **PTG CONSULTING**.

3. PROVIDE SUPPORTS TO REINFORCEMENT IN THE FORM OF PROPRIETARY CONCRETE, METAL, OR PLASTIC CHAIRS, SPACERS, HANGERS AND TIES. SPACING SHALL NOT EXCEED60x BAR DIAMETERS FOR BARS OR 800mm FOR MESH. UNCONVENTIONAL OR A-TYPICAL BAR CHAIRS / REINFORCEMENT SUPPORT SHALL BE REVIEWED BY **PTG CONSULTING** PRIOR TO USE.

4. REINFORCEMENT SUPPORTS SHALL BE ADEQUATE TO WITHSTAND ALL APPLIED LOADS, INCLUDING CONSTRUCTION AND TRAFFIC. METAL BAR CHAIRS SHALL NOT BE USED IN EXTERNAL SLABS OR SLABS WITH EXPOSURE CLASS B1 OR MORE SEVERE. USE ADEQUATE MEASURES TO PREVENT DAMAGE TO WATERPROOF MEMBRANES OR VAPOUR BARRIERS AS REQUIRED.

5. DISTRIBUTION REINFORCEMENT SHALL BE N12-450 WHERE TRANSVERSE REINFORCEMENT IS NOT SHOWN ON PLAN. ANY DISTRIBUTION REINFORCEMENT SHALL BE POSITIONED IN ACCORDANCE WITH THE BAR LAYING SEQUENCE NOTED ON PLAN.

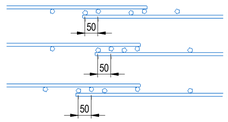
6. REINFORCEMENT IS REPRESENTED DIAGRAMMATICALLY AND NOT NECESSARILY IN TRUE PROJECTION. ALL LAPS SHALL BE IN THE SAME PLANE UNLESS SPECIFICALLY DOCUMENTED ON DRAWINGS.

7. SPLICES IN REINFORCEMENT SHALL BE MADE ONLY IN POSITION SHOWN OR OTHERWISE APPROVED IN WRITING BY THE ENGINEER. WHERE SPLICES ARE REQUIRED AND NOT SHOWN ON DRAWINGS SUBMIT PROPOSED DETAILS TO **PTG CONSULTING** FOR REVIEW.

8. **BAR LAPS** IN mm SHALL BE AS SHOWN BELOW UNLESS NOTED OTHERWISE:

LAP LENGTHS mm (U.N.O.)									
BAR DIAM.	N12	N16	N20	N24	N28	N32	N36	N40	N50
LAP LENGTH	450	700	800	1000	1200	1300	1500	1700	2100

9. MESH LAPS SHALL BE AS SHOWN BELOW:



ALL MESH LAPS SHALL BE WIRED TOGETHER AT 1000mm CENTRES.

10. **SAFETY MESH**. THE BUILDER SHALL PROVIDE SAFETY MESH TO SATISFY ANY SAFE WORKING REQUIREMENTS. POSITION SAFETY MESH IN ACCORDANCE WITH THE ADDITIONAL REINFORCEMENT NOTE. SAFETY MESH IS NOT SHOWN ON THE DRAWINGS AND IS NOT INCLUDED IN ANY REINFORCING SCHEDULES OR RATES.

11. **ADDITIONAL REINFORCEMENT**. SHOULD THE CONTRACTOR WISH TO ADD ADDITIONAL REINFORCING STEEL TO ANY DOCUMENTED STRUCTURAL ELEMENT, SUCH REINFORCEMENT SHALL BE POSITIONED SO AS NOT TO ALTER THE POSITION OR CHAIRED HEIGHT OF THE DOCUMENTED REINFORCEMENT. ADDITIONAL REINFORCING STEEL MUST COMPLY TO SPECIFIED CONCRETE COVERS AND MUST ALLOW FOR THE ADEQUATE PLACEMENT AND COMPACTION OF CONCRETE.

12. **HOT-DIP GALVANISING**. REINFORCEMENT SPECIFIED TO BE HOT-DIP GALVANISED SHALL BE UNDERTAKEN IN ACCORDANCE WITH AS/NZS 4680. PROVIDE A MINIMUM ZINC COATING OF 700G/m² AND PASSIVATION BY DIPPING INTO 0.2% SODIUM DICHROMATE SOLUTION. IF REPAIRS ARE REQUIRED TO GALVANISING: PROPOSED RECTIFICATIONS SHALL BE SUBMITTED TO **PTG CONSULTING** FOR REVIEW AND APPROVAL IN ACCORDANCE WITH **AS/NZS 4680, SECTION 8**.
ANY BENDING OF REINFORCEMENT (COGS, HOOKS, JOGGLES, ETC.) SHALL BE PERFORMED PRIOR TO GALVANISING. REINFORCING BARS WITH EVIDENCE OF BENDING AFTER GALVANISING SHALL BE SUBJECT TO REJECTION.

REINFORCING STEEL CONT'D:

13. **TRIMMERS:** ALL RE-ENTRANT CORNERS AND SERVICE PENETRATIONS SHALL HAVE TRIMMER BARS PLACED DIAGONALLY AT CORNERS USING TWO BARS (1600 LONG) WITH ONE TIED TO THE UNDERSIDE OF TOP REINFORCEMENT AND THE OTHER TIED TO THE TOP OF THE BOTTOM REINFORCEMENT. TRIMMER BARS SHALL BE N12 FOR SLABS NOT THICKER THAN 120mm, N16 FOR SLABS NOT THICKER THAN 180mm, N20 OR 2-N16 UNLESS NOTED OTHERWISE.

14. ALL BARS IN TRIMMER BAR GROUPS SHALL BE THE SAME LENGTH. SPACE BARS AT APPROXIMATELY 100mm CENTRES.

15. **JOGGLES** TO BARS SHALL BE 1x BAR DIAMETER OVER A LENGTH OF 12x BAR DIAMETERS UNLESS NOTED OTHERWISE.

16. **FACTORY BENDING** OF REINFORCEMENT, INCLUDING COGS, HOOKS, JOGGLES, ETC. SHALL BE UNDERTAKEN IN ACCORDANCE WITH **AS 3600**.

17. **SITE BENDING** OF REINFORCEMENT SHALL CONFORM WITH **AS 3600** AND THE FOLLOWING:

    a. ALL BARS SHALL BE BENT AROUND A PIN OF DIAMETER CONFORMING WITH **AS 3600** REQUIREMENTS.
    b. HEATING OF BARS TO ASSIST BENDING IS NOT PERMITTED ON SITE.
    c. BARS SHALL NOT BE BENT USING IMPACT, SUCH AS WITH HAMMERS.
    d. BENDING OF REINFORCEMENT THAT HAS BEEN HOT-DIP GALVANISED IS NOT PERMITTED.
    e. WHERE SITE RE-BENDING IS REQUIRED, THIS SHALL BE UNDERTAKEN USING A PROPER BAR BENDING TOOL AND IN ACCORDANCE WITH **AS 3600 CL 17.2.3.2.A** BAR SHALL ONLY BE RE-BENT ONCE AT THE SAME LOCATION.

18. **R GRADE DOWELS** SHALL BE IN ACCORDANCE WITH **AS/NZS 4671**. UNLESS NOTED OTHERWISE ON THE DRAWINGS, ALL R GRADE DOWELS SHALL BE GRADE 250N AND BE HOT-DIP GALVANISED, WITH THE EXPOSED END THOROUGHLY GREASED.

19. **WELDING OF REINFORCEMENT** SHALL ONLY BE UNDERTAKEN WHERE DOCUMENTED, UNLESS SUBMITTED TO AND APPROVED BY **PTG CONSULTING**. IF WELDING OF REINFORCEMENT IS PROPOSED, PROVIDE DETAILS. WHERE APPROVED, WELDS SHALL BE PERFORMED TO **AS 1554.3** AND SHALL NOT BE LOCATED:
    a. WITHIN 75mm OF A BEND WITH AN INTERNAL RADIUS < 12x BAR DIAMETERS; OR
    b. AT POINTS WHICH HAVE BEEN RE-BENT.

20. **UNENCASED REINFORCEMENT:** IF STARTER BARS AND OTHER ITEMS PROJECT FROM CAST CONCRETE FOR FUTURE ADDITIONS AND ARE EXPOSED TO THE WEATHER, PROVIDE DETAILS OF PROTECTION. ACCEPTABLE PROTECTION METHODS:
    a. HOT-DIPPED GALVANISED.
    b. ENCASED IN SLURRY.
    c. WRAPPED IN DENSO TAPE.

21. **TYING:** SECURE THE REINFORCEMENT AGAINST DISPLACEMENT BY TYING AT INTERSECTIONS WITH EITHER WIRE TIES OR CLIPS. BEND THE ENDS OF WIRE TIES AWAY FROM NEARBY FACES OF FORMS SO THAT THE TIES DO NOT PROJECT INTO THE CONCRETE COVER.

POST INSTALLED ANCHOR STUDS AND REINFORCEMENT:

1. ALL REFERENCE TO "ANCHORS" IN THIS NOTE REFERS TO BOTH ANCHOR STUDS AND POST-INSTALLED REINFORCING BARS.

2. ALL MECHANICAL AND CHEMICAL POST INSTALLED ANCHORS ARE TO COMPLY WITH THE REQUIREMENTS OF **AS 3600** AND **AS 5216** AND ARE TO BE INSTALLED STRICTLY IN ACCORDANCE WITH THE MANUFACTURER'S SPECIFICATIONS.

3. POST INSTALLED REINFORCEMENT IS NOT TO BE USED UNLESS SPECIFIED ON DRAWINGS, OR AGREED IN WRITING WITH **PTG CONSULTING**.

4. INSTALLATION SHALL BE PERFORMED BY SUITABLY QUALIFIED PERSONNEL (EITHER AN **AEFAC** CERTIFIED INSTALLER OR EQUIVALENT CERTIFICATION BY THE MANUFACTURER/SUPPLIER OF THE SPECIFIED PRODUCT).

5. UNLESS NOTED OTHERWISE, ALL ANCHOR STUDS TO BE **GRADE 5.8** HOT-DIP GALVANISED STEEL STUDS AND ALL REINFORCEMENT TO BE **N500** GRADE.

6. ALL CHEMICAL ANCHORING ADHESIVES ARE TO COMPLY WITH THE FOLLOWING:
    - ANCHOR STUDS INTO SOLID CONCRETE: **HILTI HIT-HY 200R, RAMSET CHEMSET 801 XTREM, ICCONS BIS-HY GEN2** OR APPROVED EQUIVALENT. ANCHORS TO BE INSTALLED INTO FULLY CURED CONCRETE/GROUT.
    - ANCHOR STUDS INTO CORE-FILLED BLOCKWORK: **HILTI HIT-HY 200R, RAMSET CHEMSET 801 XTREM, ICCONS BIS-P GEN2 POLYPRO** OR APPROVED EQUIVALENT. ANCHORS TO BE INSTALLED INTO FULLY CURED CONCRETE / GROUT.
    - ANCHOR STUDS INTO HOLLOW CONCRETE MASONRY OR SOLID CLAY MASONRY: **HILTI HIT-HY 270, RAMSET CHEMSET 101 PLUS, ICCONS BIS-P GEN2 POLYPRO** OR APPROVED EQUIVALENT. PROVIDE PROPRIETARY SLEEVE SYSTEM FOR HOLLOW MASONRY (**RAMSET**: NYLON SLEEVE OR FINE METAL MESH SLEEVES, **HILTI: HIT-SC SCREEN TUBE**, OR APPROVED EQUIVALENT). HOLES MUST BE DRILLED WITHOUT HAMMERING FUNCTION TO PREVENT BLOWOUT FROM THE BACK OF THE MASONRY UNIT.
    - POST-INSTALLED REINFORCING BARS INTO SOLID CONCRETE OR CORE-FILLED BLOCKWORK: **HILTI HIT-RE-500V3, RAMSET CHEMSET 801 XTREM, ICCONS BIS-PE GEN3** OR APPROVED EQUIVALENT.

7. ALL HOLES FOR ANCHORS ARE TO BE HAMMER DRILLED (WITH THE EXCEPTION OF ANCHORS INTO HOLLOW MASONRY). HAMMER DRILLING IS NOT TO BE SUBSTITUTED WITH CORE DRILLING WITHOUT PRIOR APPROVAL BY **PTG CONSULTING**.

8. ANCHORS ARE NOT TO BE INSTALLED INTO EXISTING CRACKS. CONTACT **PTG CONSULTING** FOR ADVICE IF CLASHES OCCUR WITH REQUIRED ANCHOR POSITIONS.

9. REINFORCEMENT AND POST-TENSIONED CABLES/TENDONS ARE NOT TO BE CUT OR DAMAGED IN ANY WAY BY ANCHOR INSTALLATION. THE BUILDER IS TO INCORPORATE CLASH DETECTION METHODS SUCH AS CONCRETE SCANNING AND PILOT HOLE DRILLING TO LOCATE EXISTING REINFORCEMENT/POST-TENSIONING. SHOULD CLASHES WITH EXISTING REINFORCEMENT/POST-TENSIONING BE DETECTED, CONTACT **PTG CONSULTING** FOR APPROVAL OF ALTERNATIVE POSITIONING OF ANCHORS PRIOR TO INSTALLATION. ALL PILOT HOLES ARE TO BE FILLED WITH A CEMENTITIOUS OR EPOXY GROUT FILLER APPROVED BY **PTG CONSULTING**.

10. THE CONTRACTOR SHALL ALLOW TO PERFORM AND PAY FOR ALL COSTS ASSOCIATED WITH THE TESTING OF CHEMICAL AND MECHANICAL ANCHORS. FAILURE OF ANCHORS TO PASS TEST FAILURE CRITERIA DURING TESTING WILL INCUR FURTHER TESTING AND MAY REQUIRE REPLACEMENT OF INSTALLED ANCHORS AT THE CONTRACTOR'S EXPENSE.

11. TESTING SHALL BE PERFORMED BY PERSONNEL INDEPENDENT FROM THE ANCHOR INSTALLERS. TESTING RIG CALIBRATION CERTIFICATES, TESTING METHODOLOGY, AND TESTING FAILURE CRITERIA ARE TO BE SUBMITTED AT **PTG CONSULTING'S** REQUEST.

12. PROOF TEST CHEMICAL AND MECHANICAL ANCHORS WITHIN A TEST SAMPLE POPULATION TO THE PROOF LOADS NOMINATED ON PROJECT DOCUMENTATION IN ACCORDANCE WITH THE TESTING PROCEDURE IN THE **AEFAC** TECHNICAL NOTE: SITE TESTING GUIDELINES - VOL 2. THE MINIMUM TEST LOAD DURATION SHALL BE 150 SECONDS.

13. FOR TEST LOAD MAGNITUDE REFER PROJECT DOCUMENTATION FOR PROOF LOADS. IF NO PROOF LOAD HAS BEEN NOMINATED, CONTACT **PTG CONSULTING** FOR INSTRUCTION.

14. FOR TESTING FREQUENCY WHERE TEST ANCHORS ARE NOT SPECIFICALLY NOMINATED ON DRAWINGS, THE MINIMUM NUMBER OF ANCHORS TO BE TESTED PER TEST SAMPLE POPULATION IS AS FOLLOWS (U.N.O. ON PROJECT DRAWINGS OR AGREED IN WRITING WITH THE **PTG CONSULTING**):
CHEMICAL ANCHORS, SIDE AND BELOW: 10% OF TOTAL POPULATION BUT A MINIMUM OF 5 ANCHORS CHEMICAL ANCHORS, OVERHEAD: 100% OF TOTAL POPULATION MECHANICAL ANCHORS: 10% OF TOTAL POPULATION BUT A MINIMUM OF 5 ANCHORS POST-INSTALLED REINFORCEMENT: AS PER CHEMICAL ANCHORS.

POST INSTALLED ANCHOR STUDS AND REINFORCEMENT CONT'D:

15. THE "TEST SAMPLE POPULATION' IS DEFINED AS A GROUP OF ANCHORS REPRESENTATIVE OF THE RELEVANT ANCHOR POPULATION, HAVING THE SAME:

    - ANCHOR TYPE
    - BASE MATERIAL (THAT HAS NOT EXPERIENCED DIFFERENT ENVIRONMENTAL EXPOSURE)
    - INSTALLATION METHOD
    - INSTALLATION PERSONNEL.

    WHERE ANY OF THESE VARIABLES CHANGE, THIS GROUP OF ANCHORS SHALL BE CONSIDERED AS A SEPARATE ANCHOR POPULATION. EACH ANCHOR POPULATION MUST UNDERGO TESTING IN ACCORDANCE WITH THIS NOTE.

16. IF ONE FAILURE IS ENCOUNTERED THEN THE REASON FOR FAILURE SHALL BE DETERMINED AND **PTG CONSULTING** IMMEDIATELY NOTIFIED. IF A FAILURE IS ENCOUNTERED 100% OF THE ANCHORS SHALL BE TESTED U.N.O. REPLACE FAILED FIXINGS AS PER PTG CONSULTING'S WRITTEN INSTRUCTIONS.

17. SUBMIT RESULTS OF TESTING IN THE FORM OF A TEST REPORT TO PTG CONSULTING. AT A MINIMUM, THE TEST REPORT IS TO CONTAIN THE INFORMATION NOMINATED IN **APPENDIX A** OF THE **AEFAC TECHNICAL NOTE: SITE TESTING GUIDELINES - VOL 1.** THE TEST REPORT IS TO ALSO CONTAIN PHOTOGRAPHS OF THE TESTING RIG IN POSITION, AND RESULTANT PEAK AND SUSTAINED LOADS DURING TESTING.

18. IF THE CONTRACTOR WISHES TO SUBSTITUTE ANY ANCHORS THAT HAVE BEEN DOCUMENTED AS CAST-IN WITH POST-INSTALLED ANCHORS, WRITTEN APPROVAL MUST BE OBTAINED FROM **PTG CONSULTING** PRIOR TO ANCHOR INSTALLATION.

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

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Drawing Title
GENERAL NOTES PAGE - SHEET 2

Drawn	Approved	Scale @ A1	Drawing Status
SC		1 : 1	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S00-012	B

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SCARR STREET

GENERAL STEELWORK:

1. **EXTENT OF WORK:** THE WORK SHALL CONSIST OF THE SUPPLY, SHOP DRAWING, FABRICATION AND ERECTION OF STEELWORK SHOWN ON PTG CONSULTING AND OTHER CONSULTANT DOCUMENTATION INCLUSIVE OF SECONDARY STEELWORK. THE WORKS INCLUDES SURFACE TREATMENT, STORAGE, DELIVERY TO THE SITE, STEEL TO STEEL CONNECTIONS, FIELD WELDING, BOLTS, ANCHOR BOLTS, END PLATE GROUTING AND REPAIRS TO SURFACE TREATMENT, FASTENINGS AND ALL MISCELLANEOUS ATTACHMENTS. PRIMARY STRUCTURAL ELEMENTS HAVE BEEN INDICATED ON PTG CONSULTING'S DRAWINGS. THE CONTRACTOR SHALL INCLUDE IN THE COST ALL CO-ORDINATION, DESIGN, SHOP DRAWING, FABRICATION AND INSTALLATION OF SECONDARY STRUCTURAL ELEMENTS, INCLUDING BUT NOT LIMITED TO ROOF TRUSSES, WALL FRAMING, STUDWORK, CEILING FRAMING, FASCIA SUPPORTS, GUTTER SUPPORTS, ROOF AND WALL CLADDING SUPPORTS, ARCHITECTURAL FINISHES SUPPORTS, WINDOW AND DOOR HEADS, GLAZING SUPPORTS, DOOR JAMBS AND LINTELS, TRIMMING PURLINS, UNDER PURLINS, OUTRIGGERS, HANDRAILS, BARRIERS AND BALUSTRADES, FALL ARREST HEIGHT SAFETY SYSTEMS, BRACKETS, AND CLEATS. ALL SECONDARY ELEMENTS, CONNECTIONS AND DETAILS ARE TO BE DEVELOPED AND CO-ORDINATED BY THE CONTRACTOR IN CONJUNCTION WITH CONSULTANT'S DOCUMENTATION AND SUBMITTED FOR REVIEW PRIOR TO ISSUE OF SHOP DRAWINGS. THE WORK SHALL BE CARRIED OUT IN STRICT ACCORDANCE WITH ALL PTG CONSULTING DRAWINGS AND ALL OTHER CONSULTANT SPECIFICATIONS AND DRAWINGS.

2. ALL STEELWORK SHALL BE IN ACCORDANCE WITH THE FOLLOWING STANDARDS:
   - **STEEL**
     AS/NZS 3679.1 HOT ROLLED BARS AND SECTIONS.
     AS/NZS 3679.2 WELDED SECTIONS.
     AS/NZS 3678 STRUCTURAL STEEL- HOT ROLLED PLATES, FLOOR PLATES & SLABS.
     AS/NZS 1163 STRUCTURAL STEEL HOLLOW SECTIONS.
   - **ELECTRODES**
     AS/NZS 1553 COVERED ELECTRODES FOR WELDING.
   - **MILD STEEL BOLTS**
     AS/NZS 1110 ISO METRIC PRECISION HEXAGON BOLTS AND SCREWS.
     AS/NZS 1111 ISO METRIC HEXAGON COMMERCIAL BOLTS AND SCREWS.
     AS/NZS 1112 ISO METRIC HEXAGON NUTS INCLUDING THIN NUTS, SLOTTED NUTS AND CASTLE NUTS.
   - **BRIGHT BOLTS, PRECISION BRIGHT FORGED BOLTS AND NUTS**
     AS/NZS 1110 ISO METRIC PRECISION HEXAGON BOLTS AND SCREWS.
     AS/NZS 1111 ISO METRIC HEXAGON COMMERCIAL BOLTS AND SCREWS.
     AS/NZS 1112 ISO METRIC HEXAGON NUTS INCLUDING THIN NUTS, SLOTTED NUTS AND CASTLE NUTS.
   - **WASHERS**
     FOR BOLTS SPECIFIED ABOVE:
     AS 1237 FLAT METAL WASHERS FOR GENERAL ENGINEERING PURPOSES.
     BS3410 METAL WASHERS FOR GENERAL ENGINEERING PURPOSES.

   BOLTS, NUTS, AND WASHERS:
   - **HIGH STRENGTH BOLTS**
     AS/NZS 1252 HIGH STRENGTH STEEL BOLTS WITH ASSOCIATED NUTS AND WASHERS FOR STRUCTURAL ENGINEERING
   - **WORKMANSHIP**
     AS 4100 STEEL STRUCTURESAS/NZS 1554 STRUCTURAL STEEL WELDING.
   - **COMPOSITE STEEL-CONCRETE CONSTRUCTION INCLUDING PROFILED STEEL SHEETING AND SHEAR CONNECTORS**
     AS/NZS 2327.

   SURFACE TREATMENT:
   - **SURFACE PREPARATION**
     AS 1627 METAL FINISHING - PREPARATION AND PRETREATMENT OF SURFACES.
   - **PAINT**
     AS/NZS 2312.1 PAINT COATINGSAS 3730.21 PRIMER-SOLVENT-BORNE-FOR FERROUS METALLIC SURFACES.
   - **HOT DIP GALVANISING**
     AS/NZS 4680HOT-DIP GALVANISED (ZINC) COATINGS ON FABRICATED FERROUS ARTICLES
     AS 2331METHODS OF TESTING METALLIC & RELATED COATINGS
     AS 2312.2HOT-DIP GALVANISED
   - **IN-LINE GALVANISING**
     AS/NZS 4791 HOT DIP GALVANISED (ZINC) COATINGS ON FERROUS OPEN SECTIONS, APPLIED BY AN INLINE PRICES
     AS/NZS 4792 HOT-DIP GALVANISED (ZINC) COATINGS ON FERROUS HOLLOW SECTIONS, APPLIED BY A CONTINUOUS OR A SPECIALISED PROCESS.

3. **CONSTRUCTION CATEGORY** IN ACCORDANCE WITH **AS 5131.**
   IMPORTANCE LEVEL: **2**
   SERVICE CATEGORY: **SC1**
   FABRICATION CATEGORY: **FC1**
   CONSTRUCTION CATEGORY: **CC2**

4. **STEEL GRADES**:
   STRUCTURAL MEMBERS: **REFER DRAWINGS.**

MEMBER TYPE	GRADE
ALL PLATES (AS/NZS 3679.1)	250
ROLLED SECTION (AS/NZS 3679.1)	300
WELDED SECTION (AS/NZS 3679.2)	300
SHS, RHS AND CHS (AS/NZS 1163)	250, 350 or 450
COLD FORMED PURLINS AND GIRTS (AS/NZS 1397)	G550, Z350 or Z450

GENERAL STEELWORK CONT'D:

5. **BOLTS:** MINIMUM BOLTING REQUIREMENTS AS FOLLOW:
   TIMBER TO STEEL CONNECTIONS - **M12 4.6/S** BOLTS
   COLD FORMED SECTIONS TO STEEL - **M12 4.6/S** BOLTS
   STEEL TO STEEL CONNECTIONS - **M20 8.8/S BOLTS**
   HOLD DOWN BOLTS - **GRADE 4.6/S**
   ALL BOLTS, NUTS AND WASHERS SHALL BE HOT DIPPED GALVANISED. UNLESS NOTED OTHERWISE.
   THERE IS TO BE A MINIMUM OF 2 BOLTS IN CONNECTIONS.
   ALL BOLTS ARE DESIGNED TO INCLUDE THREADS IN THE SHEAR PLANE U.N.O.
   PROVIDE EACH ANCHOR BOLT WITH 2 NUTS AND 2 OVERSIZE WASHERS WITH SUFFICIENT THREAD FOR THE LEVELLING NUT AND WASHER TO SET BELOW THE BASE PLATE.
   TB AND TF BOLTS TO BE INSTALLED USING APPROVED LOADING INDICATING WASHERS. BOLTING PROCEDURES ARE DENOTED IN THE FOLLOWING MANNER.

IDENTIFICATION MARK AS SHOWN	4.6/s	8.8/s	8.8/TB	8.8/TF
BOLT STRENGTH GRADE	4.6	8.8	8.8	8.8
BOLT TO AUSTRALIAN STANDARD	AS 1111	AS/NZS 1252	AS/NZS 1252	AS/NZS 1252
TIGHTENING SPECIFICATION	SNUG TIGHT	SNUG TIGHT	BEARING TENSIONED	FRICTION TENSIONED

   CONTRACTOR TO ENSURE CORRECT TENSIONING OF BOLTS IN ACCORDANCE WITH DRAWINGS. CONTRACTOR SHALL SUBMIT BOLT TENSIONING PROCEDURE AND RELEVANT CALIBRATION TESTS OF EQUIPMENT WHERE REQUESTED.
   ALL BOLT HOLES SHALL BE 2mm LARGER THAN THE NOMINAL BOLT DIAMETER U.N.O.
   SLOTTED HOLES FOR H.D. BOLTS SHALL BE 4mm OVERSIZE U.N.O.
   WHERE WASHER TAPERED SLEEVES ARE USED FOR RHS AND SHS CONNECTIONS, THEY MUST BE WELDED TO BOTH WALLS OF THE MEMBER.

6. U.N.O. SEAL THE ENDS OF HOLLOW SECTIONS WITH 3mm SEAL PLATE TYPICAL.

7. **WELDING:** WELD TYPE NOT SHOWN ON THE DRAWINGS TO TO HAVE A NOMINAL TENSILE STRENGTH OF 490 MPA AND ARE TO BE 6MM FILLET ALL ROUND (6 CFW).
   WELD CATEGORY TO BE S.P. U.N.O.
   WELDING OF COLD FORMED SECTIONS TO BE 3.0MM CONTINUOUS FILLET WELD, USING E70 OR E80 LOW HYDROGEN COATED ELECTRODE OR MIG EQUIVALENT.
   REMOVE OXIDISED MATERIAL AND TOUCH UP WELDS WITH ZINC RICH PAINT (ORGANIC ZINC SILICATE).
   SITE WELDS WHERE NOTED ON PLAN SHALL BE THOROUGHLY WIRE BRUSHED AND CLEANED.
   ALL WELDED IN TO BE UNDERTAKEN BY A QUILIFIED WELDER IN ACCORDANCE WITH AS1544 PART 1.
   TESTING OF WELDS SHALL BE UNDERTAKEN IN ACCORDANCE WITH AS/NZS 1554.1.
   100% OF ALL WELDS TO EXAMINED VISIUAL MEANS, 10% SP BUTT WELDS TO BE EXAMINED BY RADIOGRAPHIC OR ULTRASONIC INSPECTION, 5% OF SP FILLET WELDS TO BE COMPLETED BY MAGNETIC PARTICLE OR LIQUID PENETRATING U.N.O.

8. THE STEEL FABRICATOR SHALL PROVIDE ALL FIXINGS CLEATS AND BOLT HOLES TO TAKE TIMBER AND OTHER WORK SHOWN OR IMPLIED ON THE ARCHITECTURAL DRAWINGS.

9. REFER POST INSTALLED ANCHOR NOTES FOR ALL CHEMICAL AND MECHANICAL ANCHORS.

10. **COLD FORMED PURLINS AND GIRTS.** REFER TO THE STEELWORK DRAWINGS FOR THE MANUFACTURER AND PRODUCT TYPE USED FOR THE DESIGN. SUBSTITUTE SECTIONS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER. SECTIONS SHALL A MINIMUM GALVANISED COATING OF 450 gm/sqm. BRIDGING AND PURLIN LAP DETAILS SHALL BE IN ACCORDANCE WITH MANUFACTURERS SPECIFICATION U.N.O. ON ENGINEERING DRAWINGS. ALL CEILINGS AND BUILDING SERVICES (MECHANICAL, ELECTRICAL, FIRE, ETC.) ARE NOT TO BE FIXED TO PURLIN FLANGES UNLESS PRIOR APPROVAL IS GIVEN BY THE STRUCTURAL ENGINEER.

11. THESE DRAWINGS DETAIL SUFFICIENT INFORMATION TO SHOW THE DESIGN INTENT. THEY ARE NOT TO TAKE PLACE OF SHOP DRAWINGS WHICH SHOW ALL NECESSARY INFORMATION FOR THE FABRICATION OF EACH MEMBER.

12. THE STEEL FABRICATOR IS TO PROVIDE THE STRUCTURAL ENGINEER WITH DIGITAL SHOP DRAWINGS FOR INSPECTION BEFORE FABRICATION BEGINS.

13. STEELWORK IS TO BE CLEANED OF ALL RUST, MILLSCALE, GREASE ETC. TO AN **AS 1627 CLASS 1** PREPARATION, AND GIVEN A 0.05mm COAT OF RED OXIDE ZINC PHOSPHATE BEFORE DISPATCH TO THE SITE, UNLESS THE STEEL IS TO BE ENCASED OR IS DETAILED OTHERWISE. STEELWORK NOTED TO BE HOT DIPPED GALVANISED SHALL BE CLEANED IN ACCORDANCE WITH **AS 1627 PART 5**. THEN HOT DIPPED GALVANISED IN ACCORDANCE WITH **AS 4680** TO GIVE AN AVERAGE ZINC COATING WEIGHT OF 600 gm/sqm (MINIMUM ZINC COATING WEIGHT OF 550 gm/sqm

GENERAL STEELWORK CONT'D:

14. STEELWORK EXPOSED TO THE WEATHER (ie. A STEEL ELEMENT NOT TOTALLY CLAD IN ITS FINAL POSITION AND HAVING SOME PORTION OF ITS SURFACE EXPOSED TO AIR, WHETHER INDOORS, UNDERCOVER OR FULLY EXPOSED TO THE ELEMENTS) AND NOT NOTED AS BEING HOT DIPPED GALVANISED SHALL BE CLEANED OF ALL RUST, MILLSCALE, GREASE ETC. TO AN **AS 1627 CLASS 2.5** SURFACE PREPARATION, AND GIVEN A COAT OF SELF CURING ETHYL INORGANIC ZINC SILICATE OF A TYPE 4 PAINT TO **AS 3750**. ALL PAINTING SHALL BE IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS AND SHALL HAVE AN AVERAGE DRY FILM THICKNESS OF 75 MICRONS. THE MINIMUM DRY FILM THICKNESS AT ANY POINT SHALL NOT BE LESS THAN 65 MICRONS.

15. REPAIR GALVANISING WHERE DAMAGED BY WELDING AS FOLLOWS: MODERATE CORROSION - 2 COATS OF "JOTUN GALVANITE" EPOXY RICH ZINC PRIMER TO 125-150 MICRON DFT) IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATION. SEVERE CORROSION - 2 COATS OF "DULUX ZINCANODE 402" ZINC PRIMER TO 150 MICRON FOLLOWED BY 2 PACK EPOXY ENAMEL TO 150 MICRON IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

16. **ERECTION AND INSTALLATION:** ERECTION OF STEELWORK SHALL CONFORM TO THE REQUIREMENTS OF AS/NZS 5131, REFER SAFETY AND DESIGN NOTES.
    THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING AN ERECTION PROCEDURE REPORT.
    ALL MEMBERS MUST BE MAINTAINED IN THEIR INTENDED VERTICAL AND LATERAL ALIGNMENT AND LEVEL TO MEET SPECIFIED TOLERANCES.
    MEMBERS WHICH DO NOT MEET THE TOLERANCES SPECIFIED IN CLAUSE 15.3 OF AS4100 SHALL BE LIABLE TO REJECTION.
    THE STEELWORK MUST BE SECURED AGAINST WIND, ERECTION STRESSES, AND LOADING CONDITIONS DURING ERECTION.
    PERMANENT BOLTING OR WELDING SHOULD NOT BE PERFORMED UNTIL CORRECT ALINGMENT AND CAMBER HAVE BEENACHIEVED IN EACH MEMBER.
    THIS PROCESS SHOULD BE FOLLOWED PROGRESSIVELY FOR MULTI-STOREY BUILDINGS.
    THE CONTRACTOR MUST ENSURE THE STABILITY OF THE STRUCTURE THROUGHOUT THE ERECTION PROCESS. ANY ADDITIONAL MEMBERS USED FOR ERECTION MUST NOT WEAKEN OR DEFACE PERMANENT STEELWORK.

17. THE ROOF STRUCTURE HAS BEEN DESIGNED TO WITHSTAND LOADS AND DESIGN ACTIONS AS SPECIFIED IN THE RELEVANT AUSTRALIAN STANDARDS **AS 1170** AND PROPRIETARY DESIGN INFORMATION PROVIDED BY ROOF SHEETING AND ROOF PURLIN MANUFACTURERS. THE ROOF STRUCTURE HAS NOT BEEN DESIGNED TO RESIST LOADS IMPOSED FROM FALL ARREST ANCHORS. THE CONTRACTOR IS TO ENGAGE THE SERVICES OF A RPEQ TO PROVIDE CERTIFICATION THAT THE ROOF STEELWORK HAS SUFFICIENT CAPACITY TO RESIST THE LOADS IMPARTED FROM ANY PROPOSED FALL ARREST SYSTEM. IT IS THE CONTRACTOR'S RESPONSIBILITY TO PROVIDE ANY ADDITIONAL STEELWORK REQUIRED TO SUPPORT THE PROPOSED SYSTEM.

18. **INSPECTION:** PROVIDE AT LEAST 48 HOURS NOTICE SO THAT INSPECTION MAY BE MADE OF ALL STRUCTURAL STEELWORK ERECTED ON SITE (PRIOR TO GROUTING, ANCASING, SITE PAINTING OR CLADDING)

19. **MATERIAL TEST CERTIFICATES:** PRIOR TO STEEL ERECTION SUBMIT DOCUMENTATION WHICH DEMONSTRATES THAT ALL THE STEEL COMPLIES WITH THE SPECIFIED MATERIAL STANDARDS AND IS SUITABLE FOR FABRICATION OF AUSTRALIAN STANDARDS. CERTIFIED MILL TEST REPORTS, OR TEST CERTIFICATES ISSUED BY TESTING AUTHORITIES WITH 3rd PARTY ACCREDITATION RECOGNISED BY ILAC (MRA), IN CONFORMANCE WITH AS/NZS 1163 GRADE LO FOR COLD FORMED HOLLOW SECTIONS, AS/NZS 3679.1 FOR HOT ROLLED BARS OR SECTIONS OR AS/NZS 3679.2 FOR WELDED I SECTIONS, AS/NZS 1252.1 SECTION 6 FOR BOLTS (INCLUDINGLOCAL NATA-ACCREDITED LABORATORY COMPLIANCE CERTIFICATE).

    A) STRUCTURAL STEEL (PLATES, ROLLED AND WELDED SECTIONS, SHS, RHS, CHS)
    - STEEL SUPPLIER DETAILS.
    - CERTIFIED MILL TEST REPORTS OR TEST CERTIFICATES CONFIRMING COMPLIANCE WITH RELEVANT AUSTRALIAN STANDARDS.

    B) BOLTS AND BOLT ASSEMBLY (BOLTS, NUTS, WASHERS)
    - BOLT AND BOLT ASSEMBLY SUPPLIER DETAILS.
    - CERTIFIED TEST REPORTS OR TEST CESTIFICATES CONFIRMING COMPLIENCE WITH RELEVANT AUSTRALIAN STANDARDS.
    - BOLT TRACEABILITY MARKINGS.

    C) PURLINS ANG GIRTS
    - PURLIN AND GIRT SUPPLIER DETAILS.
    - CERTIFIED MILL TEST REPORTS OR TEST CERTIFICATES CONFIRMING COMPLIANCE WITH AS 1397.

    FURTHER TO ABOVE ENSURE BORON CONTENT IS NOMINATED ON DRAWINGS AND IS LESS THAN 0.0008%.
    DELIVERY DOCKETS FOR EACH BATCH/PORTION/DELIVERY FOR ALL STEEL RECEIVED BY THE FABRICATOR FOR THE PROJECT, WITH REFERENCE TO THE ASSOCIATED HEAT CERTIFICATES. ON REQUEST OF THE PROJECT ENGINEER, THE FABRICATOR MAY BE REQUIRED TO SUBMIT RELEVANT DATA IN ALTERNATIVE FORMAT SUCH AS A SPREADSHEET.
    IF MATERIAL CERTIFICATES CANNOT BE PROVIDED, CONTACT PTG CONSULTING FOR REQUIRED TESTING PROCEDURES.

GENERAL STEELWORK CONT'D:

20. **PURCHASE AND TRACEABILITY:** THE PROCUREMENT, FABRICATION AND ERECTION OF STRUCTURAL STEELWORK SHALL BE UNDERTAKEN UNDER A DOCUMENTED COMPLIANCE MANAGEMENT PLAN (COMPMP). THE COMPMP SHALL INCLUDE THE REQUIREMENTS OF THE QUALITY PLAN IN AS/NZS 5131. ALL STRUCTURAL STEEL SHALL BE SOURCED FROM MILLS WITH A RELEVANT JAS ANZ ACCREDITED THIRD PARTY CERTIFICATION SCHEME SUCH AS THE ACRS SCHEME. ALTERNATIVE SOURCING OF THIRD PARTY CERTIFIED STRUCTURAL STEEL SHALL BE SUBMITTED FOR REVIEW AND MUST BE APPROVED PRIOR TO THE COMMENCEMENT OF PROCUREMENT. STEELWORK SHALL BE FABRICATED BY FABRICATORS CERIFIED UNDER THE ASI 'NATIONAL STRUCTURAL STEELWORK COMPLIANCE SCHEME' (NSSCS). HIGH STRENGHT STRUCTURAL BOLTS SHALL BE VERIFIED TO AS/NZS 1252.2. LEVEL OF TRACEABILITY SHALL BE AS DEFINED IN AS/NZS 5121 CLAUSE 4.7.

21. **SHOP DRAWING REVIEW:** THE CONTRACTOR IS RESPONSIBLE FOR PREPARING AND SUBMITTING TWO COPIES (IF REQUIRED) OF ALL SHOP DRAWINGS TO PTG CONSULTING FOR REVIEW BEFORE THE STARTING FABRICATION. THE SUPERINTENDENT DOES NOT TAKE RESPONSIBILITY FOR THE ACCURACY OR PRODUCTION OF THE CONTRACTOR'S SHOP DRAWINGS. ANY DELAYS OR COSTS ASSOCIATED WITH SHOP DRAWING RFI'S, RFI RESPONSES, AND REVISIONS OF SHOP DRAWINGS ARE THE RESPONSIBILITY OF THE CONTRACTOR.
    THE SHOP DRAWINGS MUST EXHIBIT CLEAR AND COMPLETE DETAILS OF EACH ASSEMBLY, COMPONENT, AND CONNECTION IN THE WORK, INCLUDING INFORMATION RELATED TO THEIR FABRICATION, SURFACE TREATMENT, AND ERECTION. THE CONTRACTOR MUST ALSO INCLUDE CONNECTION DETAILS THAT ARE NOT INDICATED ON THE STRUCTURAL DRAWING FOR REVIEW BEFORE ISSUING THE SHOP DRAWINGS. THE CONTRACTOR MUST COORDINATE WITH ALL CONSULTANT DRAWINGS AND SECONDARY STEELWORK FOR SHOP DETAILER CO-ORDINATION, AND THEY MUST CONFIRM THAT ALL MEMBERS CAN BE ERECTED CORRECTLY.
    UNLESS OTHERWISE AGREED UPON, THE SHOP DRAWING REVIEW WILL TAKE PLACE WITHIN SEVEN (7) WORKING DAYS OF RECEIVING THE DRAWINGS, COVERING MEMBER SIZES, SURFACE TREATMENT, AND STRUCTURAL CONNECTION SOUNDNESS. THE REVIEW WILL NOT INCLUDE DIMENSIONS, HOLES, CLEATS, OR OTHER ITEMS. ALL COMMENTS MADE ON THE SHOP DRAWINGS MUST BE ADDRESSED OR CORRECTED BEFORE FABRICATION.

22. **SITE MODIFICATION:** APPROVAL MUST BE OBTAINED BEFORE CUTTING, BURNING, WELDING, OR DRILLING STEEL MEMBERS DURING ERECTION.

23. **STORAGE AND HANDLING:** HANDLE COMPONENTS AND MEMBERS WITH CARE TO PREVENT OVERSTRESSING, DEFORMATION, OR DAMAGE TO THEIR PROTECTIVE COATING. ANY DAMAGED ITEMS SHOULD BE RECTIFIED OR REPLACED, AND APPROVAL SHOULD BE OBTAINED BEFORE ASSEMBLING THEM INTO THE STRUCTURE. TO AVOID DAMAGE TO SURFACE FINISHES DURING HANDLING AND ERECTION, WRAP OR PROTECT THE MEMBERS OR COMPONENTS. STORE THEM OFF THE GROUND AND AVOID DIRECT CONTACT BETWEEN STEEL SLINGS AND COATED STEELWORK WHEN USING LIFTING POINTS.

24. **GROUTING:** STEELWORK SUPPORTED BY CONCRETE, MASONRY, OR SIMILAR MATERIAL SHOULD BE PLACED ON PACKING OR WEDGES TO ALLOW ALLIGNMENT AND FUTURE GROUTING. PERMANENT PACKS SHOULD BE MADE OF SOLID STEEL OR GROUT WITH SIMILAR STRENGHT TO PERMIT GROUT.
    NON-PERMANENT PACKS MUST BE REMOVED BEFORE GROUTING IS FINISHED. GROUTING SHOULD HAVA A MINIMUM STRENGHT OF 50MPa AT 7 DAYS U.N.O. GROUTING TO BE IN ACCORDANCE WITH AS/NZS 5131 CLAUSE 5.8.

25. **BEAM CAMBER:** IF BEAM MEMBERS HAVE NATURAL CAMBER WITHIN THE STRAIGHTNESS TOLERANCE, FABRICATE AND ERECT THEM WITH THE CAMBER UP.

26. **CONNECTIONS:** CONTRACTOR TO MAKE ALLOWANCE FOR ALL MEMBER CONNECTIONS - ALL CONNECTIONS TO BE BASED ON ASI STRUCTURAL STEEL - SIMPLE CONNECTIONS U.N.O.
    FOR ANY CONNECTIONS NOT DETAILED ON ENGINEERING DRAWINGS CONTRACTOR TO SUBMIT PROPOSAL FOR PTG CONSULTING TO REVIEW IN ACCORDANCE TO DESIGN INTENT.
    IF SPLICES ARE REQUIRED BY THE CONTRACTOR WHERE NOT SHOWN ON ENGINEERING PLANS, CONTRACTOR TO PROPOSE DETAILS AND LOCATIONS FOR REVIEW, ANY ADDITIONAL COST ASSOCIATED WITH APLICING SHALL BE AT THE CONTRACTOR'S EXPENSE.

27. **STANDARD REQUIREMENTS:** VERTICAL SUPPORT IS REQUIRED TO HEAD AND SILL MEMBERS AT MID-SPAN IN THE FORM OF HANGERS OR POSTS WHERE SPANS EXCEED 4m.
    BRACING MEMBERS SHALL BE SCREW FIXED OR SIMILARLY HUNG FROM PURLINS AT 3m MAX CTRS.
    PROPIETARY ITEMS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS SPECIFICATIONS.
    ALL SUSPENDED CEILINGS ARE TO COMPLY WITH THE REQUIREMENTS OF AS 2785.2000
    PURLIN SPACING SHOWN ON DRAWINGS ARE A MAXIMUM, THEY ARE TO BE VARIED AS REQUIRED TO SUIT ROOF SHEETING, ETC.
    ALL FIXINGS TO PURLING SHALL BE WEB FIXED IN ACCORDANCE WITH PURLIN MANUFACTURERS SPECIFICATIONS AND RECOMMENDATIONS.
    THE CONTRACTOR SHALL ENSURE THAT PURLINS ARE NOT OVERLOADED OR DISTORTED FROM THE FIXINGS OF SERVICES, CEILINGS WALKWAY, ROOF SAFETY SUPPORT SYSTEM, PLANT, ROOF-MOUNTED PLATFORMS, ETC.
    THE CONTRACTOR SHALL PROVIDE CERTIFICATION FROM A REGISTERED ENGINEER FOR THE EARTHQUAKE ASSESSMENT AND FASTENING OF ALL NON-STRUCTURAL PARTS AND COMPONENTS IN COMPLIANCE WITH AS1170.4-2007 PART 8.

GENERAL STEELWORK CONT'D:

28. **PROTECTIVE COATINGS:** ALL STEEL COMPONENTS MUST BE PROTECTED AGAINST ATMOSPHERIC CORROSION USING COATINGS THAT COMPLY WITH AS/NZS 2312.1 AND AS/NZS 2312.2. ALL EXTERNAL STEELWORK SHALL BE HOT-DIP GALVANISED U.N.O.

	YEARS TO FIRST MAINTENANCE	ATMOSPHERIC CORROSIVITY CATEGORY
INTERNAL	15 TO 25 YEARS	C2
EXTERNAL	15 TO 25 YEARS	C3

    STEELWORK WITHOUT SURFACE TREATMENT MUST BE FREE OF LOOSE RUST, MILL SCALE, DIRT, OIL, AND GREASE.
    NO TREATMENT IS REQUIRED FOR STEEL WORK ENCASED IN CONCRETE. COMPONENTS TO BE SITE WELDED AND CONTACT SURFACES IN FRICTION TYPE JOINTS MUST BE KEPT CLEAR OR SPECIFIED FINISH. AFTER WELDING OR BOLTING, THE FINISH MUST BE APPLIED FOR COMPLETE COVERAGE.

29. **MINIMUM PROTECTING COATING:** UNLESS SPECIFIED OTHERWISE, ALL STEELWORK MUST BE BLAST CLEANED TO CLASS 2 1/2 IN ACCORDANCE WITH AS 1627 AND RECEIVE ONE COAT OF RED OXIDE ZINC PHOSPHATE PRIMER. THE THICKNESS OF THE PRIMER MUST COMPLY WITH THE MANUFACTURER'S RECOMMENDATIONS, AND MINIMUM DRY FILM THICKNESS MUST BE 0.075mm. INORGANIC ZINC SILICATE PRIMER (IZS1 OR IZS2 TO AS 2312.1) MUST COMPLY WITH AS 3730.21. STEEL SURFACES MUST BE DRY WHEN APPLYING PAINT, AND ALTERNATIVE PRIMERS MAY BE CONSIDERED BASED ON CORROSION RESISTANCE INFORMATION.

30. **HOT-DIP GALVANISING:** PRIOR TO GALVANISING, SURFACES MUST BE CLEANED OF ALL DIRT, WELD SPATTER, GREASE, SLAB, OIL, PAINT, OR OTHER DELETETIOUS MATERIALS.
    STEEL SURFACES MUST BE CHEMICALLY DESCALED OR ABRASIVE BLAST CLEANED TO CLASS 3 IN ACCORDANCE WITH AS 1627. THE ZINC COATING MUST BE A UNIFORM LAYER OF COMMERCIALLY PURE ZINC, FREE OF IMPERFECTIONS, AND FIRMLY ADHERING TO THE SURFACE OF THE STEEL. THE QUALITY AND THICKNESS OF GALVANISING MUST COMPLY WITH AS 2312.2 AND AS/NZS 4680. THE THICKNESS OF GALVANISING MUST BE DETERMINED USING SITE CORROSIVITY CATEGORY AND YEARS TO FIRST MAINTENANCE NOTED ON THE CONSTRUCTION DRAWINGS. THE WEIGHT ON STEEL SECTIONS, PLATES, AND TUBES MUST BE DETERMINED IN ACCORDANCE WITH AS 2331. ALL EXTERNAL STEELWORK MUST BE HOT-DIP GALVANISED U.N.O. ON THE DRAWINGS.

31. **PROTECTIVE COATING REPAIRS:** SURFACE TREATMENT DAMAGE MUST BE REPAIRED CONSISTENTLY WITH THE ORIGINAL SURFACE PREPARATION AND TREATMENT. FOR REPAIRS TO HOT-DIP GALVANISED STEEL OR SITE WELDING, PROTECTION MUST BE REPAIRED IN ACCORDANCE WITH CL7.3.2.2 OF AS 2312.2:2014.

32. **STEELWORK BELOW GROUND:** ALL STRUCTURAL STEELWORK IN CONTACT OR BELOW GROUND LEVEL TO BE PAINTED WITH 'FOSROC NICOTE EP410' AND ENCASED WITH 80mmTHICK CONCRETE ALL AROUND.

33. **EXPOSED STEELWORK:** REFER ARCHITECT FOR FINISH DETAILS TYPICALLY. ATTENTION TO FINISH SHALL BE GIVEN TO EXPOSED OR ARCHITECTURAL STEELWORK. WHEN USING SHEARING, FLAME CUTTING, OR CHIPPING METHODS, THEY MUST BE DONE WITH GREAT CARE AND PRECISION. CORNERS AND EDGES MUST BE SHARP AND FREE FROM DAMAGE CAUSED BY HANDLING OR ASSEMBLY.

REV	DATE	DESCRIPTION	DRAWN	CHK
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Drawing Title
GENERAL NOTES PAGE - SHEET 3

Drawn	Approved	Scale @ A1	Drawing Status
SC		1 : 1	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S00-013	B

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SCARR STREET

ASPECT OF DESIGN	POTENTIAL HAZARD / RISK	RISK ANALYSIS			RISK MITIGATION STRATEGY	RESIDUAL RISK ANALYSIS		
		CONSEQUENCE	LIKELIHOOD	LEVEL OF RISK		CONSEQUENCE	LIKELIHOOD	LEVEL OF RISK
SITE OR FACILITY ACCESSIBILITY	CONSTRUCTION EQUIPMENT ACCESS, MATERIALS DELIVERY HANDLING AND ERECTION. NARROW AND STEEP STREET FOR ALL SITE ACCESS.	**MODERATE**	**LIKELY**	**HIGH**	ACCESS TO SITE TO BE CLEARLY SEPARATED FROM PUBLIC AREAS WITH USE OF HOARDING / FENCES IN ACCORDANCE WITH COUNCIL REGULATIONS. UTILISE STOP / GO TRAFFIC SYSTEM DURING SITE DELIVERIES / CONCRETE TRUCKS AND DELIVERIES.	**MINOR**	**UNLIKELY**	**LOW**
DEMOLITION	DUST CREATION DURING EXCAVATION, DANGER OF COLLAPSING STRUCTURES.	**MAJOR**	**POSSIBLE**	**EXTREME**	ENSURE DUST SUPPRESSION eg WATER SPAYING DURING EXCAVATION. ALL DUE CARE TO BE TAKEN TO ENSURE STABILITY OF REMAINING STRUCTURE.	**MINOR**	**UNLIKELY**	**LOW**
	EXISTING STRUCTURE NOT EXPECTED TO CONTAIN ASBESTOS				**N/A**			
TEMPORARY WORKS	INSTABILITY OF EXISTING GROUND DURING CONSTRUCTION.	**MODERATE**	**POSSIBLE**	**HIGH**	ENSURE QUALIFIED TEMPORARY WORKS ENGINEER IS ENGAGED TO ENSURE ADEQUATE PROPPING AND STABILITY OF STRUCTURE IS MAINTAINED DURING EXCAVATION AND / OR CONSTRUCTION.	**MODERATE**	**UNLIKELY**	**MEDIUM**
EXCAVATIONS FOR SERVICES, FOUNDATIONS AND BUILDING	DAMAGE TO EXISTING UNDERGROUND SERVICES. TRIP HAZARDS. OPEN EXCAVATION COLLAPSE.	**MAJOR**	**LIKELY**	**EXTREME**	CONFIRM LOCATION OF ALL EXISTING UNDERGROUND SERVICES eg DIAL-BEFORE-YOU-DIG. ENSURE ADEQUATE FENCING AROUND ALL OPEN EXCAVATIONS AND THAT EXCAVATIONS OR SUITABLY STABLE / BATTERED OR PROPPED.	**MODERATE**	**UNLIKELY**	**MEDIUM**
HAZARDOUS SUBSTANCES	NO UNUSUAL OR ATYPICAL CONSTRUCTION RELATED SUBSTANCES PRESENT. CONCRETE POURING, PAINTING, EPOXY RESINS AND ADHESIVES.	**MINOR**	**UNLIKELY**	**LOW**	ENSURE ALL EPOXIES, PAINTS, ADHESIVES ETC. ARE USED IN ACCORDANCE WITH MANUFACTURERS RECOMMENDATIONS.	**MINOR**	**UNLIKELY**	**LOW**
SAFETY ISSUES RELATED TO CONSTRUCTION	TRIPS ON SITE WHILE FIXING REINFORCEMENT OR PLACING CONCRETE	**MODERATE**	**POSSIBLE**	**HIGH**	THE PRINCIPAL CONTRACTOR SHALL ASSESS THE REQUIREMENT FOR ADDITIONAL REINFORCEMENT STEEL IN ORDER TO PROVIDE A SAFE WORKPLACE.	**MINOR**	**UNLIKELY**	**LOW**
	STEEL FABRICATION INVOLVING CUTTING, WELDING & HANDLING HEAVY MATERIALS	**MAJOR**	**POSSIBLE**	**EXTREME**	PROVIDE ADEQUATE TRAINING TO THOSE IN DIRECT CONTACT, ENSURE APPROPRIATE PPE WORN AT ALL TIMES, & IMPLEMENT PROPER SAFETY PROTOCOLS	**MINOR**	**UNLIKELY**	**LOW**
	MATERIAL HANDLING OF HEAVY STEEL FRAMES	**MAJOR**	**POSSIBLE**	**EXTREME**	THE PRINCIPAL CONTRACTOR SHALL PROVIDE THE USE OF MECHANICAL AIDS, AND OPERATED BY ADEQUATELY TRAINED OPERATORS	**MINOR**	**UNLIKELY**	**LOW**
	TRAFFIC CONTROL FOR IMPEDING TRAFFIC	**MAJOR**	**POSSIBLE**	**EXTREME**	DETOUR LOCAL TRAFFIC AWAY FROM CONSTRUCTION SITE. IMPLEMENT PROPER TRAFFIC CONTROL MEASURES SUCH AS SIGNAGE, BARRICADES & OPERATORS.	**MINOR**	**UNLIKELY**	**LOW**
	IMPALEMENT FROM EXPOSED ENDS OF REINFORCEMENT	**MODERATE**	**POSSIBLE**	**HIGH**	PROVIDE PROTECTIVE END CAPS TO ALL EXPOSED REINFORCEMENT THAT PROJECT BEYOND REINFORCEMENT MATS OR CAGES THROUGH FORMWORK OR FROM CAST CONCRETE UNTIL ALL REINFORCEMENT IS INCORPORATED INTO SUBSEQUENT WORKS	**MINOR**	**UNLIKELY**	**LOW**
	FALLING HAZARD NEAR SLAB PENETRATIONS AND VOIDS AS WELL AS AROUND PERIMETER OF UPPER LEVELS	**MAJOR**	**POSSIBLE**	**EXTREME**	THE PRINCIPAL CONTRACTOR SHALL PROVIDE SUITABLE SAFETY MEASURES, INCLUDING FALL ARREST BARRIERS AROUND ALL PENETRATIONS AND VOIDS. A CERTIFIED FALL ARREST AND ACCESS SYSTEM eg SCAFFOLDING SHALL BE INSTALLED AROUND THE PERIMETER OF THE CONSTRUCTION.	**MINOR**	**UNLIKELY**	**LOW**
	STEEL ERECTION	**MAJOR**	**POSSIBLE**	**EXTREME**	FOR THE ERECTION OF STEELWORK THE CONTRACTOR SHALL ENGAGE A REGISTERED PROFESSIONAL ENGINEER OF QUEENSLAND (RPEQ) TO UNDERTAKE AND EXECUTE THE ROLE OF 'DESIGNER' AS DEFINED IN **AS 3828** (REFER STEELWORK NOTES). ENSURE ADEQUATE FALL PROTECTION IS UTILISED DURING ALL STEELWORK ERECTION.	**MINOR**	**UNLIKELY**	**LOW**
DESIGN FEATURES ESSENTIAL TO SAFE SITE OPERATION	NO UNUSUAL OR ATYPICAL FEATURES HAVE BEEN IDENTIFIED.				**N/A**			
SAFETY ISSUES RELATED TO ONGOING BUILDING MAINTENANCE	ROOF ACCESS DURING CLEANING OR REPAIR TO ROOF & FALL FROM HEIGHTS	**MAJOR**	**POSSIBLE**	**EXTREME**	WE HAVE ASSUMED THE SITE/FACILITY OWNER/CONTRACTOR WILL ENGAGE THE SERVICES OF AN APPROPRIATELY QUALIFIED SAFE ACCESS CONSULTANT TO REVIEW THE REQUIRED SAFE ACCESS PROVISIONS FOR WORKING AT HEIGHTS AND ROOFS.	**MINOR**	**UNLIKELY**	**LOW**
	WE HAVE NOT RECEIVED ANY SPECIFIC REQUIREMENTS RELATING TO THE MAINTENANCE OF THE SITE/FACILITY. AS SUCH, OUR ASSESSMENT HAS BEEN BASED ON MAINTENANCE PROCEDURES OF SIMILAR SITES/FACILITIES AND THEIR USE. IF THERE ARE SPECIFIC REQUIREMENTS, THESE SHALL BE FORWARDED TO THE STRUCTURAL ENGINEER FOR OUR CONSIDERATION.				**N/A**			

MEASUREMENT OF LIKELIHOOD OF A WORKPLACE INCIDENT AGAINST POSSIBLE CONSEQUENCES

LIKELIHOOD	HOW LIKELY IS IT TO OCCUR?	CONSEQUENCES				
		INSIGNIFICANT	MINOR	MODERATE	MAJOR	EXTREME
		NO TREATMENT REQUIRED	FIRST AID TREATMENT ONLY, CONTAINED AT SITE	MEDICAL TREATMENT CONTAINED BUT, WITH EXTERNAL ASSISTANCE	EXTENSIVE INJURIES, LOSS OF PRODUCTION	DEATH, IRREVERSIBLE INJURIES
ALMOST CERTAIN	EXPECTED IN MOST CIRCUMSTANCES	**HIGH**	**HIGH**	**EXTREME**	**EXTREME**	**EXTREME**
LIKELY	WILL OCCUR IN MOST CIRCUMSTANCES	**MEDIUM**	**HIGH**	**HIGH**	**EXTREME**	**EXTREME**
POSSIBLE	MIGHT OCCUR AT SOME TIME	**LOW**	**MEDIUM**	**HIGH**	**EXTREME**	**EXTREME**
UNLIKELY	COULD OCCUR AT SOME TIME	**LOW**	**LOW**	**MEDIUM**	**HIGH**	**EXTREME**
HIGHLY UNLIKELY	MAY OCCUR ONLY IN EXCEPTIONAL CIRCUMSTANCES	**LOW**	**LOW**	**MEDIUM**	**HIGH**	**HIGH**

LEVEL OF RISK KEY	
LOW	MANAGED BY ROUTINE PROCEDURES
MEDIUM	HAZARD IDENTIFICATION PLANNING REQUIRED WITH PROJECT MANAGER AND CONSULTANT TEAM INVOLVEMENT IN PREVENTION
HIGH	HAZARD IDENTIFICATION RESEARCH AND PLANNING REQUIRED WITH CLIENT AND PROJECT MANAGER AND CONSULTANT TEAM INVOLVEMENT IN PREVENTION
EXTREME	HAZARD IDENTIFICATION DETAILED RESEARCH AND PLANNING REQUIRED WITH CLIENT AND PROJECT MANAGER AND CONSULTANT TEAM INVOLVEMENT IN PREVENTION

HIERARCHY OF CONTROLS			
MOST EFFECTIVE	**ELIMINATE:**		
	1.	ELIMINATE THE HAZARD (REMOVE COMPLETELY FROM WORKPLACE)	IF NOT REASONABLY PRACTICAL, THEN;
	MINIMIZE:		
	2.	SUBSTITUTE THE HAZARD (WHOLLY OR PARTLY WITH A SAFER ALTERNATIVE)	MINIMISE RISK AS REASONABLY PRACTICAL
	3.	ISOLATE THE HAZARD (USING PHYSICAL BARRIERS, TIME OR DISTANCE)	
	4.	USE PHYSICAL CONTROLS (ADAPT TOOLS AND/OR EQUIPMENT TO REDUCE RISK)	
	4.	USE ADMINISTRATIVE CONTROLS (DEVELOP SAFE METHODS OF WORK, PROCESSES AND PROCEDURES)	IF A RISK REMAINS THEN RISK MUST BE REDUCED AS FAR AS REASONABLY PRACTICAL
LEAST EFFECTIVE	5.	USE PERSONAL PROTECTIVE EQUIPMENT (PPE) (LAST OPTION AFTER CONSIDERING ALL THE ABOVE)	IF A RISK REMAINS THEN RISK MUST BE REDUCED BY USING PPE

SAFETY IN DESIGN

1. THE PURPOSE OF THIS SAFETY IN DESIGN NOTE IS TO IDENTIFY POTENTIAL HEALTH AND SAFETY HAZARDS ASSOCIATED WITH THIS PROJECT, ENGINEERING DESIGN ELEMENTS DURING ITS CONSTRUCTION, OPERATIONAL LIFE, MAINTENANCE, AND DE-COMMISSION.
2. AS THE DESIGNER FOR THE STRUCTURAL ENGINEERING ASPECTS OF THIS PROJECT, **PTG CONSULTING** HAS USED OUR BEST ENDEAVOURS TO PERFORM A RISK ASSESSMENT TO WHEREVER REASONABLY PRACTICABLE IDENTIFY ANY PORTION OF THE WORKS THAT CANNOT BE CONSTRUCTED IN A MANNER USING CURRENT TYPICAL CONSTRUCTION TECHNIQUES CONSIDERED BY THE INDUSTRY TO BE SAFE.
3. THE RISK ASSESSMENTS HAVE BEEN BASED UPON THE BEST INFORMATION AVAILABLE AT THE TIME THE PROJECT WAS DESIGNED, AND HAVE ASSUMED:
   a. THAT THE BUILDER AND SUBCONTRACTORS ARE SUFFICIENTLY COMPETENT AND ADEQUATELY TRAINED TO IDENTIFY AND ADDRESS RISKS THAT WOULD NORMALLY BE EXPECTED ON A CONSTRUCTION SITE.
   b. THAT THE END USERS OF THE SITE/FACILITY ARE ADEQUATELY TRAINED IN ITS SAFE USE AND MAINTENANCE, AND THAT ANY SPECIFIC REQUIREMENTS THAT THE SITE/FACILITY OWNER/MANAGER OR THE END USERS HAVE REGARDING THE SAFE MAINTENANCE AND USE OF THE SITE/FACILITY HAVE BEEN PROVIDED TO US FOR CONSIDERATION DURING THE DESIGN PROCESS.
   c. THAT THIS REPORT IS READ IN CONJUNCTION WITH THE INFORMATION PROVIDED BY OTHER DESIGNERS.
4. OUR RISK ASSESSMENT HAS IDENTIFIED ISSUES, LISTED IN THE ADJACENT TABLE, WHICH WE BELIEVE ARE EITHER SPECIFIC TO THIS SITE/FACILITY, UNUSUAL OR ATYPICAL. NOTWITHSTANDING THE ABOVE, WE ARE RELYING ON THE CONTRACTOR AND SUBCONTRACTORS AS EXPERTS IN THEIR RESPECTIVE FIELDS, TO ASSESS THE WORKS TO BE CARRIED OUT AND ADVISE IF THERE ARE ANY ISSUES REGARDING SAFETY ASSOCIATED WITH THE PROCUREMENT, MANUFACTURE AND INSTALLATION OF THE WORKS THAT ARE NOT LISTED IN THE ADJACENT TABLE.
5. THESE NOTES IN NO WAY RELIEVE THE PRINCIPAL CONTRACTOR OR ANY OTHER PARTY OF THEIR OWN OBLIGATIONS AND RESPONSIBILITIES UNDER THE **WORK HEALTH & SAFETY ACT 2011 QLD**, INCLUDING, BUT NOT LIMITED TO, CONSULTATION WITH THE DESIGNER UNDER **SECTION 46** OF THE ACT, THE PREPARATION OF SATISFACTORY SAFE WORK METHOD STATEMENTS AND DUTIES OF CARE. THE CONTRACTOR/OPERATOR SHALL UNDERTAKE THEIR OWN SAFETY IN DESIGN ASSESSMENTS PRIOR TO COMMENCING WORKS.
6. SAFETY DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR AND INCLUDES, BUT IS NOT LIMITED TO:
   a. DEEP EXCAVATIONS
   b. EXISTING OVERHEAD & UNDERGROUND SERVICES,
   c. WORKS NEAR ROADS, RAILWAYS
   d. PUBLIC SAFETY: PEDESTRIANS & VEHICLES IN AND AROUND THE SITE
   e. OPEN WATERBODIES, EXCAVATIONS, FLOOR PENETRATIONS
   f. GROUND SUPPORT REQUIREMENTS FOR CONSTRUCTION EQUIPMENT
7. IT IS A REQUIREMENT UNDER **SECTIONS 47 & 48** OF THE **WORK HEALTH AND SAFETY ACT 2011 QLD**, THAT A COPY OF THIS REPORT BE PROVIDED TO THE CONTRACTOR & SUBCONTRACTORS BY THE ENTITY COMMISSIONING THE WORK SHOWN ON THESE DRAWINGS AND THAT THE RELEVANT INFORMATION IS SHARED WITH THEIR WORKERS.
8. **PENETRATIONS.** REFER TO THE PRINCIPAL CONTRACTOR FOR ANY ADDITIONAL AND / OR ALTERNATIVE PENETRATION SAFETY REQUIREMENTS.
   WHERE POSSIBLE, EMPLOY CONSTRUCTION METHODOLOGIES THAT DO NOT REQUIRE OR LIMIT PENETRATIONS WHILE ENSURING THAT ALTERNATE SOLUTIONS DO NOT INTRODUCE ADDITIONAL OR INCREASED LEVELS OF RISK.
   a. ALL PENETRATION COVER PLATES ARE TO BE BRIGHTLY PAINTED RED, ORANGE OR YELLOW.
   b. HORIZONTAL PENETRATION COVER PLATES TO BE CLEARLY SIGNED "DANGER PENETRATION BELOW"
   c. VERTICAL PENETRATION COVER PLATES TO BE CLEARLY SIGNED "DANGER PENETRATION BEYOND"
   d. OBTAIN CERTIFICATION FROM THE TEMPORARY WORKS ENGINEER IF USE OF CONSTRUCTION EQUIPMENT IS REQUIRED NEAR OR OVER PENETRATION COVER PLATES.

   FOR PENETRATIONS OUTSIDE THE LIMITS OF THE PENETRATION SAFETY DETAILS, PROVIDE CERTIFIED SAFETY BARRIERS TO ALL ACCESSIBLE SIDES OR OBTAIN SPECIFIC ADVICE AND CERTIFICATION FROM THE TEMPORARY WORKS ENGINEER.
   REFER TO **PTG CONSULTING** FOR THE FILLING IN OF ALL PENETRATIONS.

Queensland Government

WorkSafe.qld.gov.au

WorkCover QUEENSLAND

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
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Drawing Title
SAFETY IN DESIGN NOTES

Drawn	Approved	Scale @ A1	Drawing Status
SC		1 : 1	PRELIMINARY
Designed	**Date**	**Project No.**	**Dwg No.**	**Revision**
JD	16/04/2024	PTG/00093	S00-111	B

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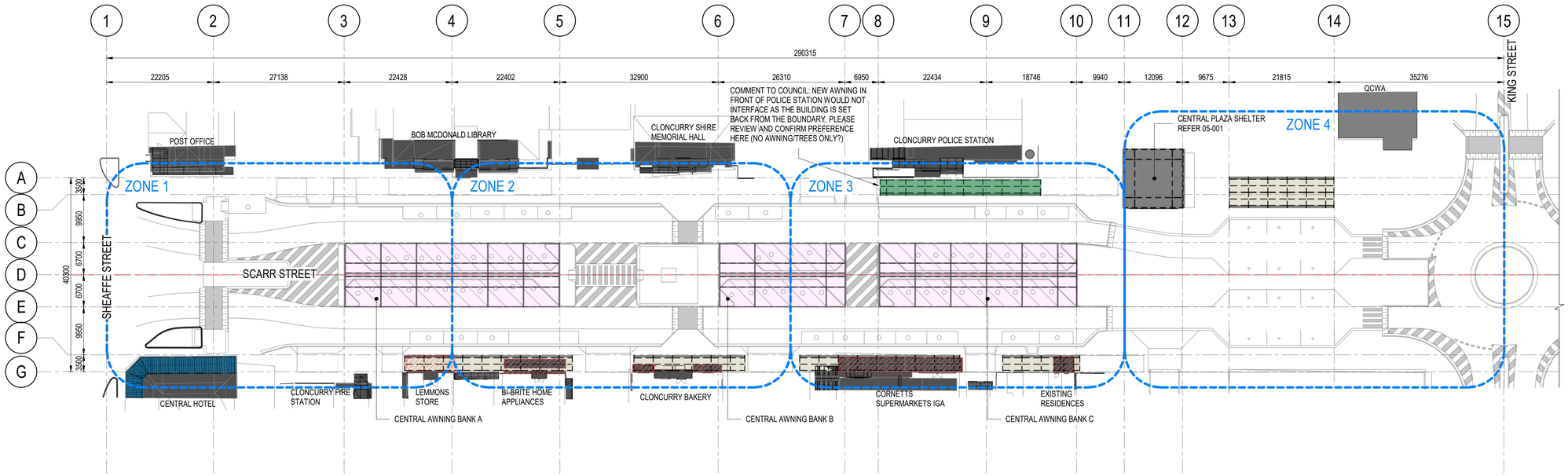

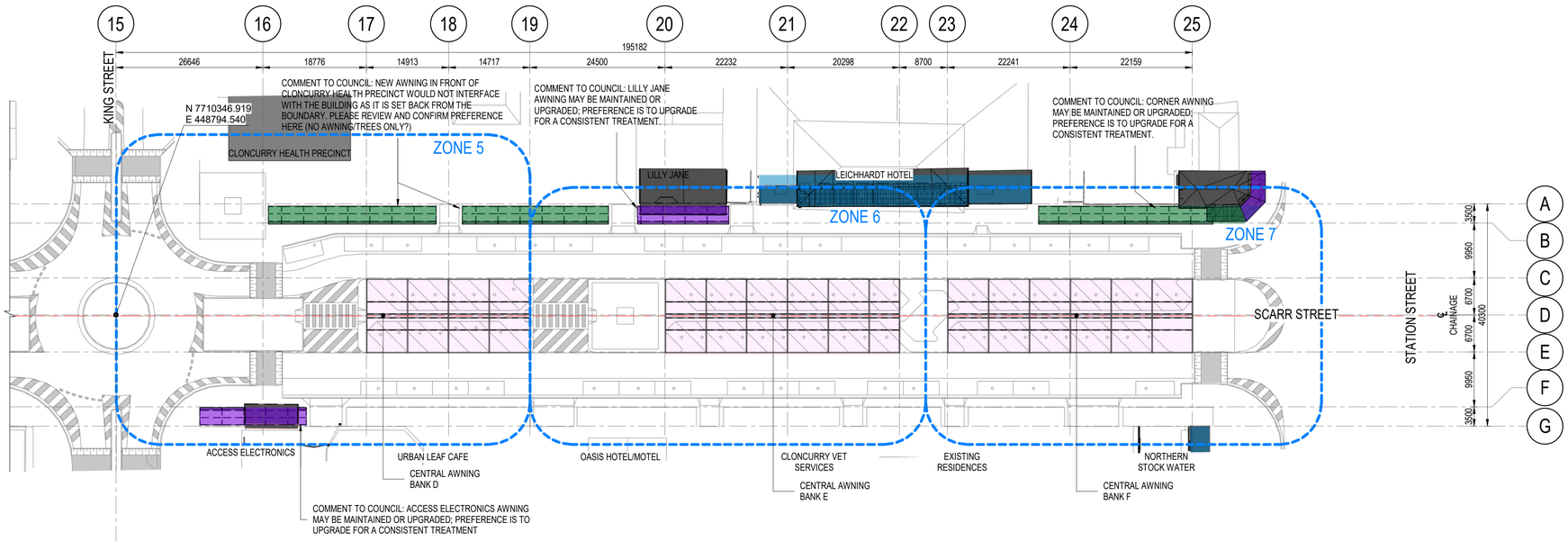

AWNING TREATMENT KEY

ALL TO BE CONFIRMED BY CLONCURRY COUNCIL

- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
- PROPOSED NEW FOOTPATH AWNING - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK
- EXISTING AWNING TO BE MAINTAINED
- EXISTING AWNING TO BE MAINTAINED OR UPGRADED - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK

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Drawing Title

OVERALL SITE LAYOUT AND MARKING PLAN

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-001	B

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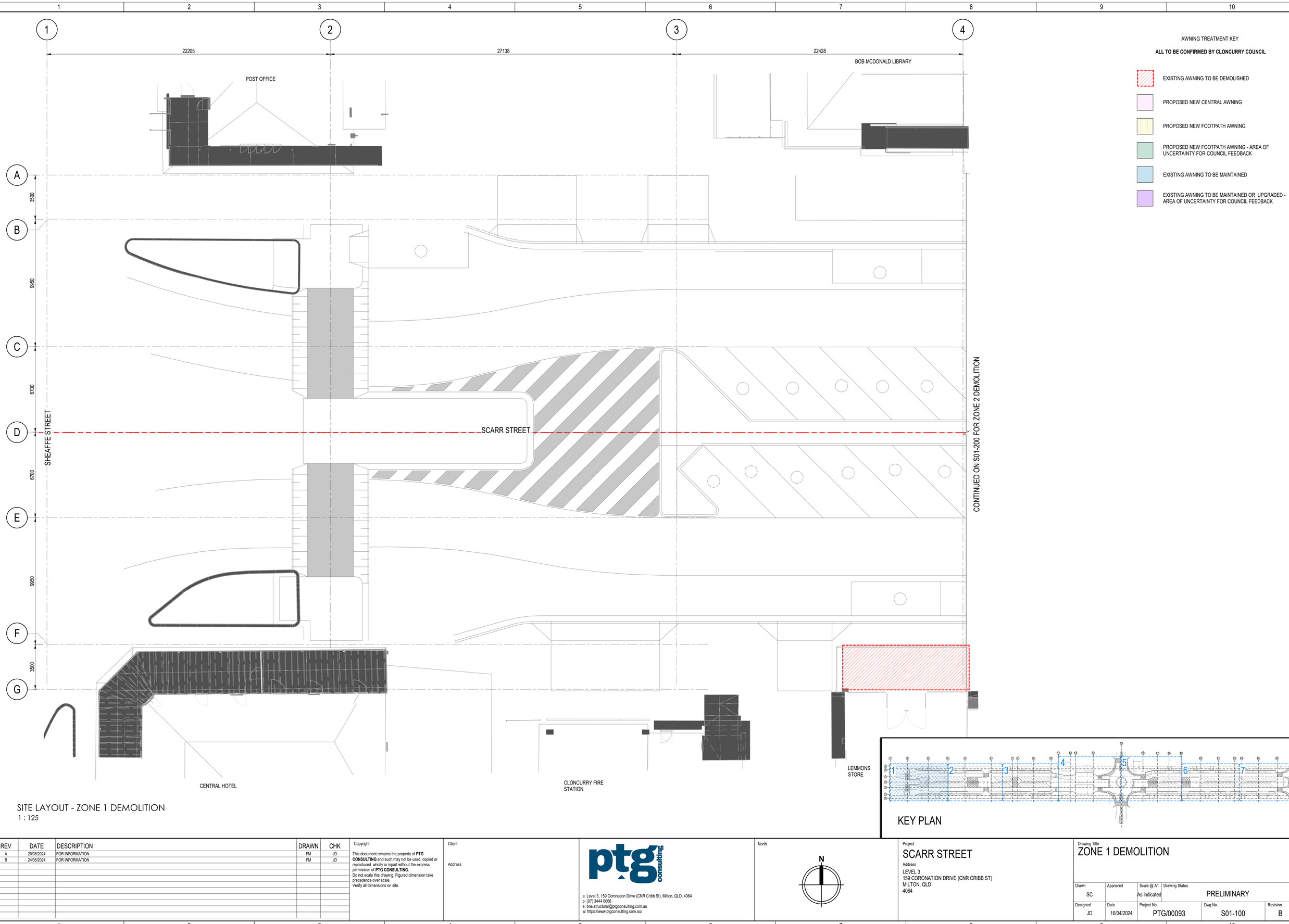

SITE LAYOUT - ZONE 1 DEMOLITION
1 : 125

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KEY PLAN

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Drawing Title
ZONE 1 DEMOLITION

Drawn	Approved	Scale @ A1	Drawing Status
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Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-100	B

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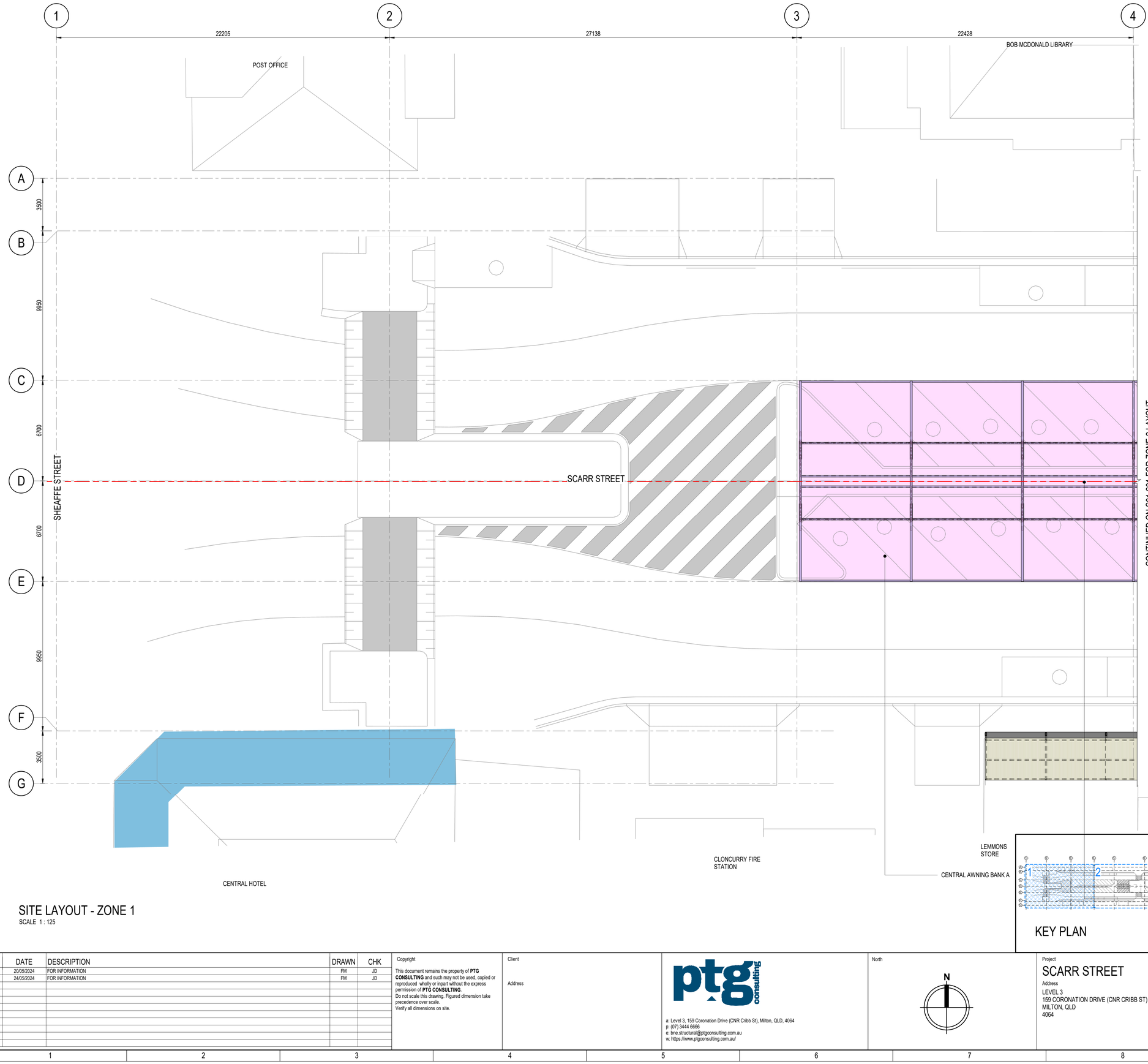

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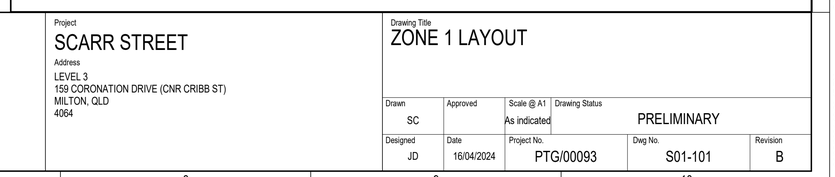

KEY PLAN

SITE LAYOUT - ZONE 1

SCALE 1 : 125

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
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Drawing Title

ZONE 1 LAYOUT

Drawn	Approved	Scale @ A1	Drawing Status
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JD	16/04/2024	PTG/00093	S01-101	B

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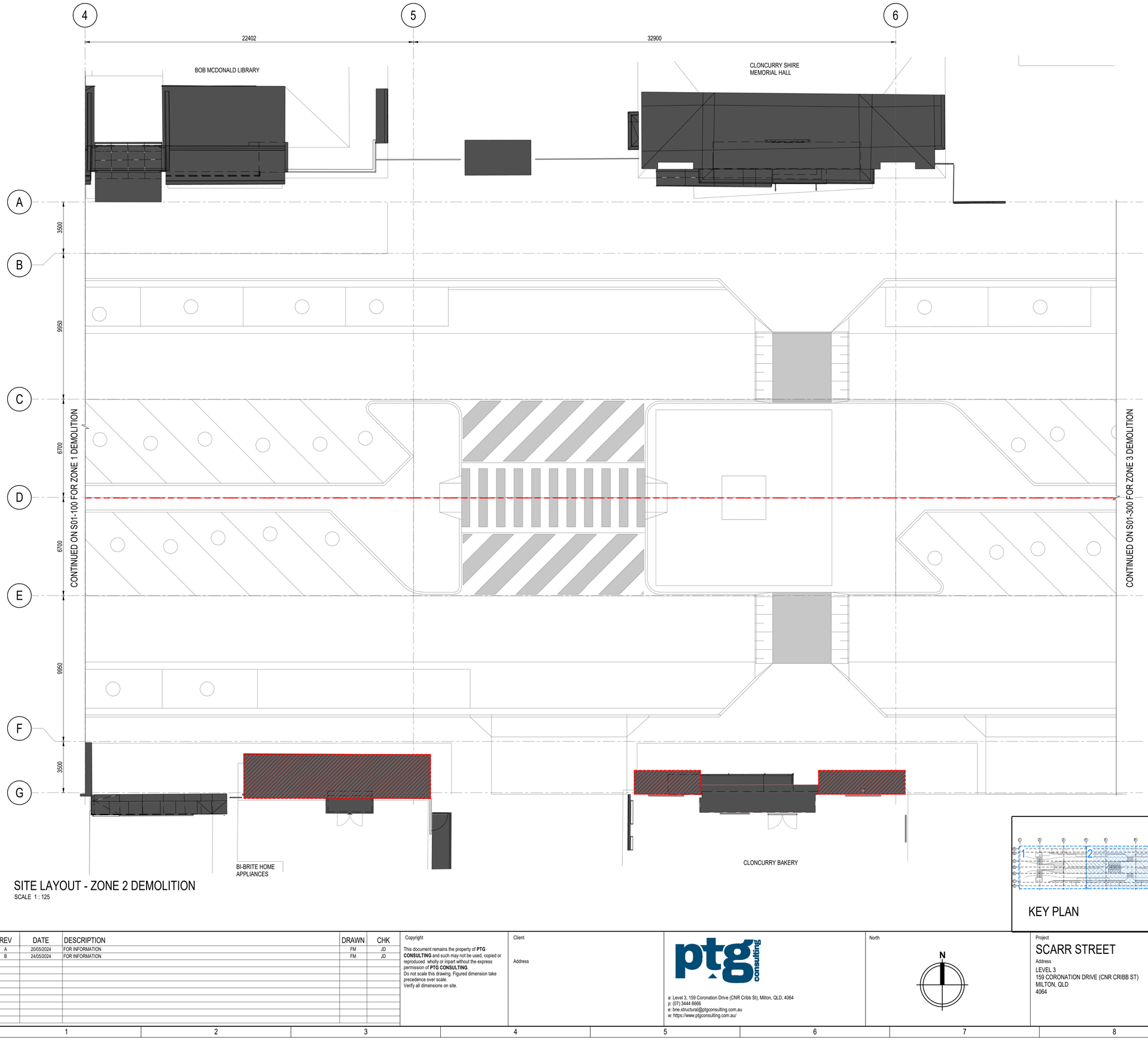

SITE LAYOUT - ZONE 2 DEMOLITION

SCALE 1 : 125

AWNING TREATMENT KEY

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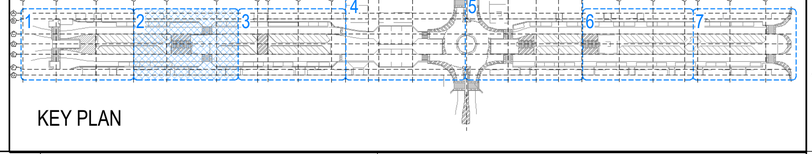

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e: bne.structural@ptgconsulting.com.au
w: https://www.ptgconsulting.com.au/

North

N

Project

SCARR STREET

Address

LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title

ZONE 2 DEMOLITION

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY
Designed	**Date**	**Project No.**	**Dwg No.**	**Revision**
JD	16/04/2024	PTG/00093	S01-200	B

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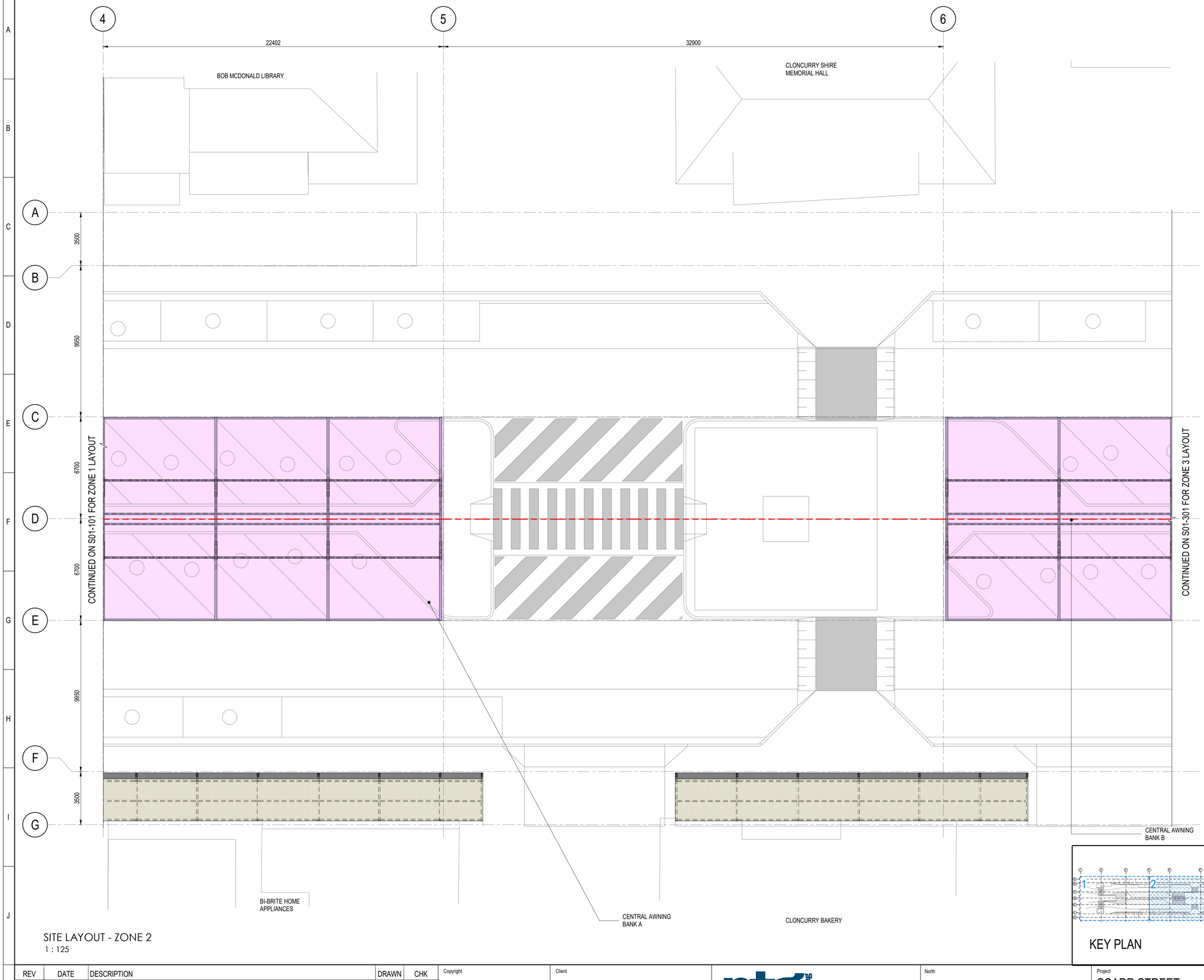
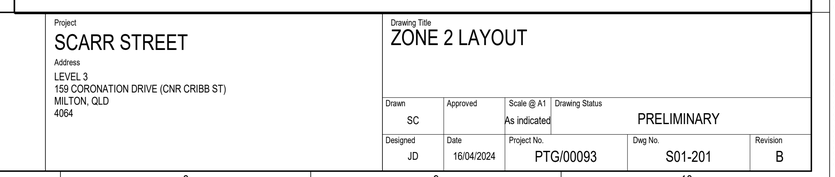


SITE LAYOUT - ZONE 2
1 : 125

AWNING TREATMENT KEY

ALL TO BE CONFIRMED BY CLONCURRY COUNCIL

- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
- PROPOSED NEW FOOTPATH AWNING - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK
- EXISTING AWNING TO BE MAINTAINED
- EXISTING AWNING TO BE MAINTAINED OR UPGRADED - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK



KEY PLAN

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

Client

Address

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a: Level 3, 159 Coronation Drive (CNR Cribb St), Milton, QLD, 4064
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North

N

Project
SCARR STREET
Address
LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title
ZONE 2 LAYOUT

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY
Designed	**Date**	**Project No.**	**Dwg No.**	**Revision**
JD	16/04/2024	PTG/00093	S01-201	B

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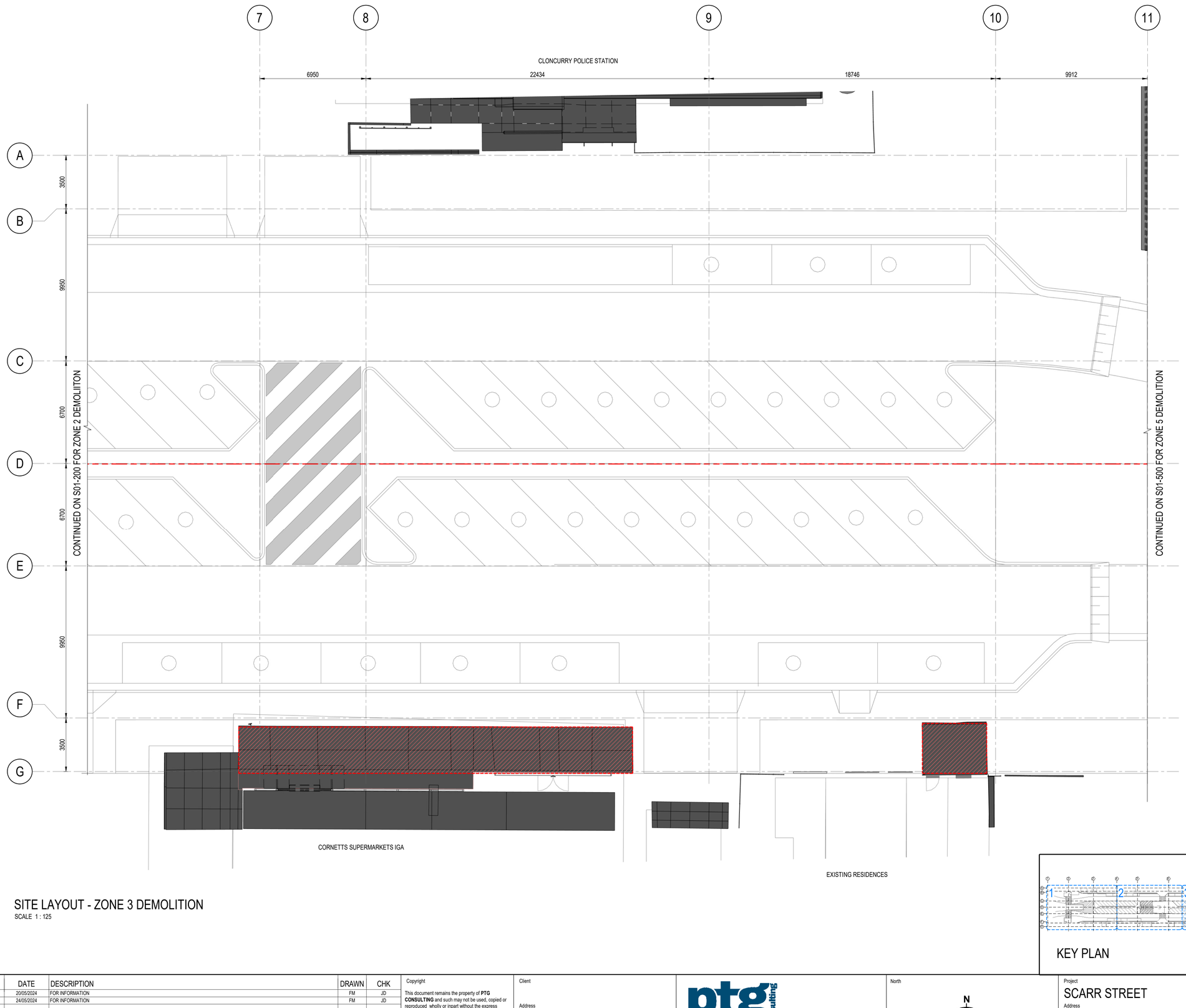

AWNING TREATMENT KEY

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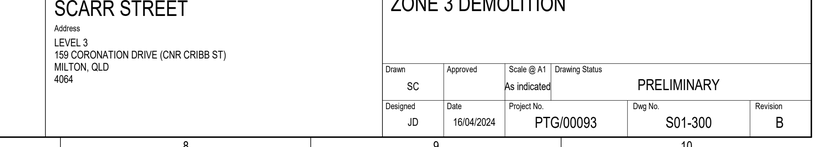
- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
- PROPOSED NEW FOOTPATH AWNING - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK
- EXISTING AWNING TO BE MAINTAINED
- EXISTING AWNING TO BE MAINTAINED OR UPGRADED - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK



KEY PLAN

SITE LAYOUT - ZONE 3 DEMOLITION

SCALE 1 : 125

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

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Address

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North

Project

SCARR STREET

Address

LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title

ZONE 3 DEMOLITION

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY
Designed	**Date**	**Project No.**	**Dwg No.**	**Revision**
JD	16/04/2024	PTG/00093	S01-300	B

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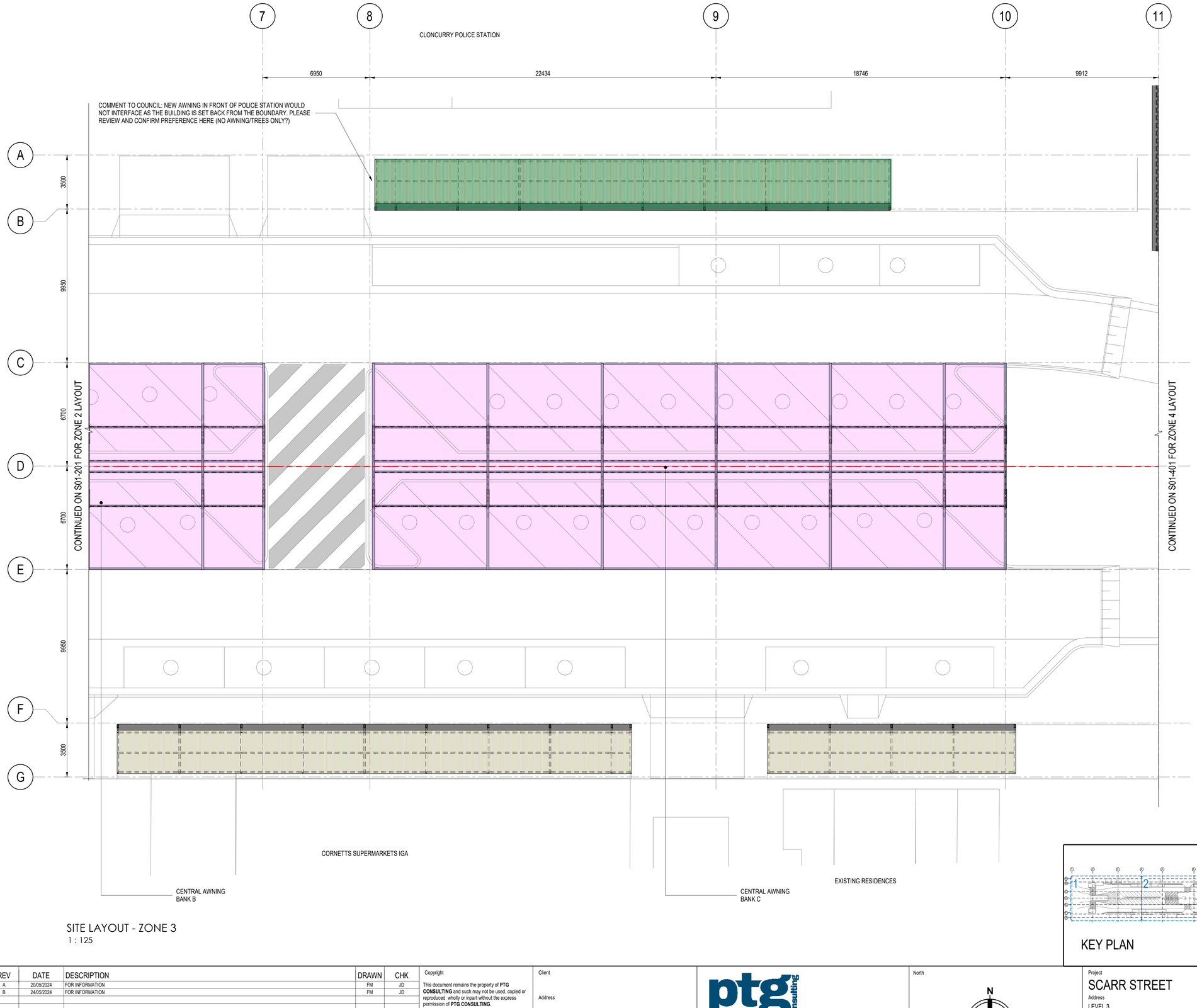
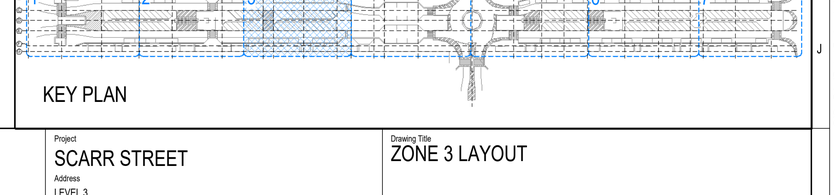


SITE LAYOUT - ZONE 3
1 : 125

AWNING TREATMENT KEY

ALL TO BE CONFIRMED BY CLONCURRY COUNCIL

- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
- PROPOSED NEW FOOTPATH AWNING - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK
- EXISTING AWNING TO BE MAINTAINED
- EXISTING AWNING TO BE MAINTAINED OR UPGRADED - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK



KEY PLAN

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

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Project
SCARR STREET
Address
LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title
ZONE 3 LAYOUT

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-301	B

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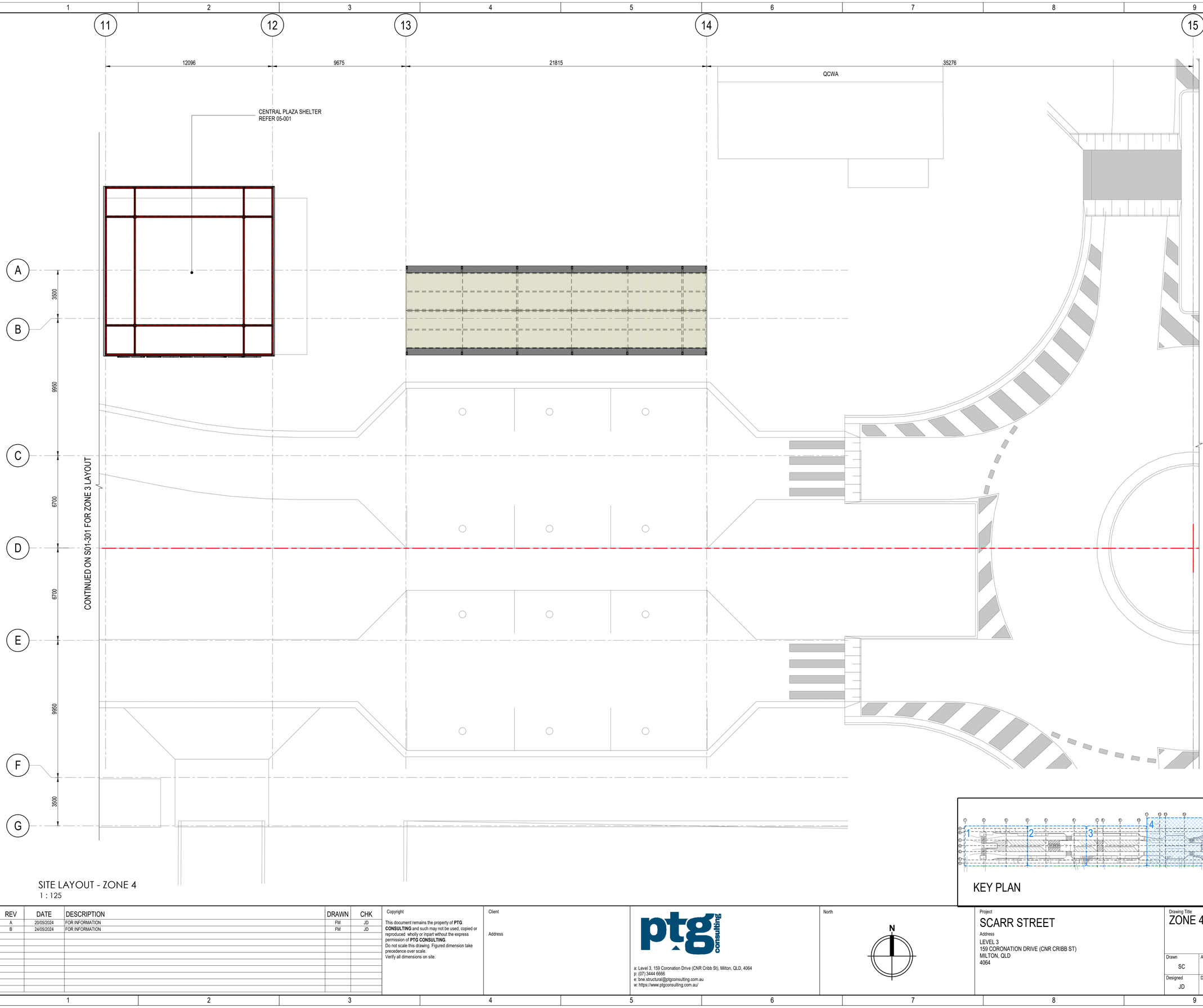

AWNING TREATMENT KEY

ALL TO BE CONFIRMED BY CLONCURRY COUNCIL

- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
- PROPOSED NEW FOOTPATH AWNING - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK
- EXISTING AWNING TO BE MAINTAINED
- EXISTING AWNING TO BE MAINTAINED OR UPGRADED - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK

SITE LAYOUT - ZONE 4
1 : 125

KEY PLAN

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

Client

Address

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North

N

Project
SCARR STREET
Address
LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title
ZONE 4 LAYOUT

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-401	B

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24/05/2024 12:10:24 PM

N 7710346.919
E 448794.540

26646 | 18776 | 14913 | 14717

CLONCURRY HEALTH PRECINCT

3500 | 9950 | 6700 | 6700 | 9950 | 3500

CONTINUED ON S01-300 FOR ZONE 3 DEMOLITION

CONTINUED ON S01-600 FOR ZONE 6 DEMOLITION

ACCESS ELECTRONICS

CENTRAL AWNING
BANK D

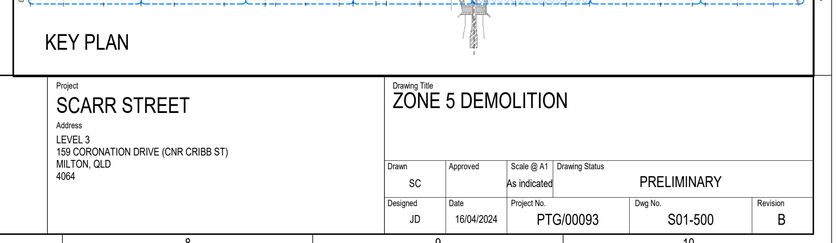
URBAN LEAF CAFE

AWNING TREATMENT KEY

ALL TO BE CONFIRMED BY CLONCURRY COUNCIL

- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
- PROPOSED NEW FOOTPATH AWNING - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK
- EXISTING AWNING TO BE MAINTAINED
- EXISTING AWNING TO BE MAINTAINED OR UPGRADED - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK

KEY PLAN



SITE LAYOUT - ZONE 5 DEMOLITION
1 : 125

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

Client

Address
CENTRAL HOTEL

ptg consulting

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w: https://www.ptgconsulting.com.au/

North

Project
SCARR STREET
Address
LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title
ZONE 5 DEMOLITION

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-500	B

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24/05/2024 12:10:37 PM

N 7710346.919
E 448794.540

26646 | 18776 | 14913 | 14717

CLONCURRY HEALTH PRECINCT

COMMENT TO COUNCIL: NEW AWNING IN FRONT OF CLONCURRY HEALTH PRECINCT WOULD NOT INTERFACE WITH THE BUILDING AS IT IS SET BACK FROM THE BOUNDARY. PLEASE REVIEW AND CONFIRM PREFERENCE HERE (NO AWNING/TREES ONLY?)

3500 | 9950 | 6700 | 6700 | 9950 | 3500

CONTINUED ON S01-401 FOR ZONE 4 LAYOUT

CONTINUED ON S01-601 FOR ZONE 6 LAYOUT

ACCESS ELECTRONICS

COMMENT TO COUNCIL: ACCESS ELECTRONICS AWNING MAY BE MAINTAINED OR UPGRADED; PREFERENCE IS TO UPGRADE FOR A CONSISTENT TREATMENT

CENTRAL AWNING BANK D

URBAN LEAF CAFE

AWNING TREATMENT KEY

ALL TO BE CONFIRMED BY CLONCURRY COUNCIL

- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
- PROPOSED NEW FOOTPATH AWNING - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK
- EXISTING AWNING TO BE MAINTAINED
- EXISTING AWNING TO BE MAINTAINED OR UPGRADED - AREA OF UNCERTAINTY FOR COUNCIL FEEDBACK

KEY PLAN

SITE LAYOUT - ZONE 5
1 : 125

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

Client
Address
CENTRAL HOTEL

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a: Level 3, 159 Coronation Drive (CNR Cribb St), Milton, QLD, 4064
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North

Project
SCARR STREET
Address
LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title
ZONE 5 LAYOUT

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-501	B

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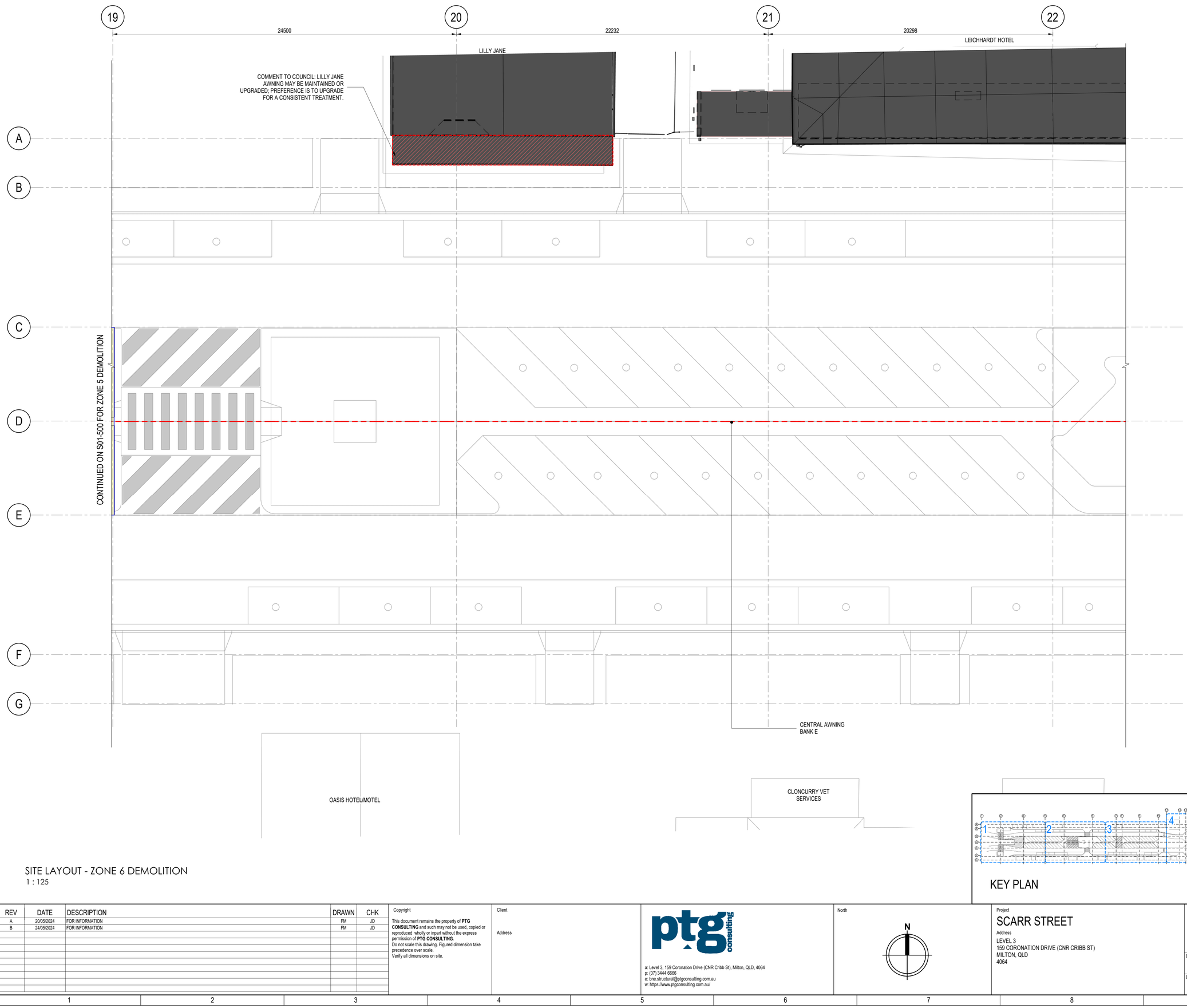

AWNING TREATMENT KEY

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- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
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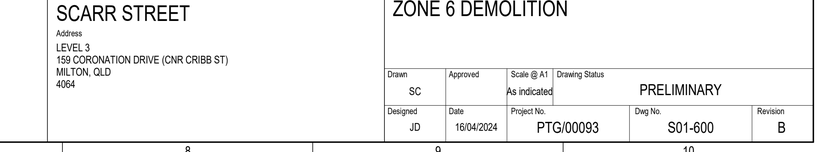
AWNING TREATMENT KEY

SCALE 1 : 1

SITE LAYOUT - ZONE 6 DEMOLITION

1 : 125



KEY PLAN

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

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Project

SCARR STREET

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LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title

ZONE 6 DEMOLITION

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-600	B

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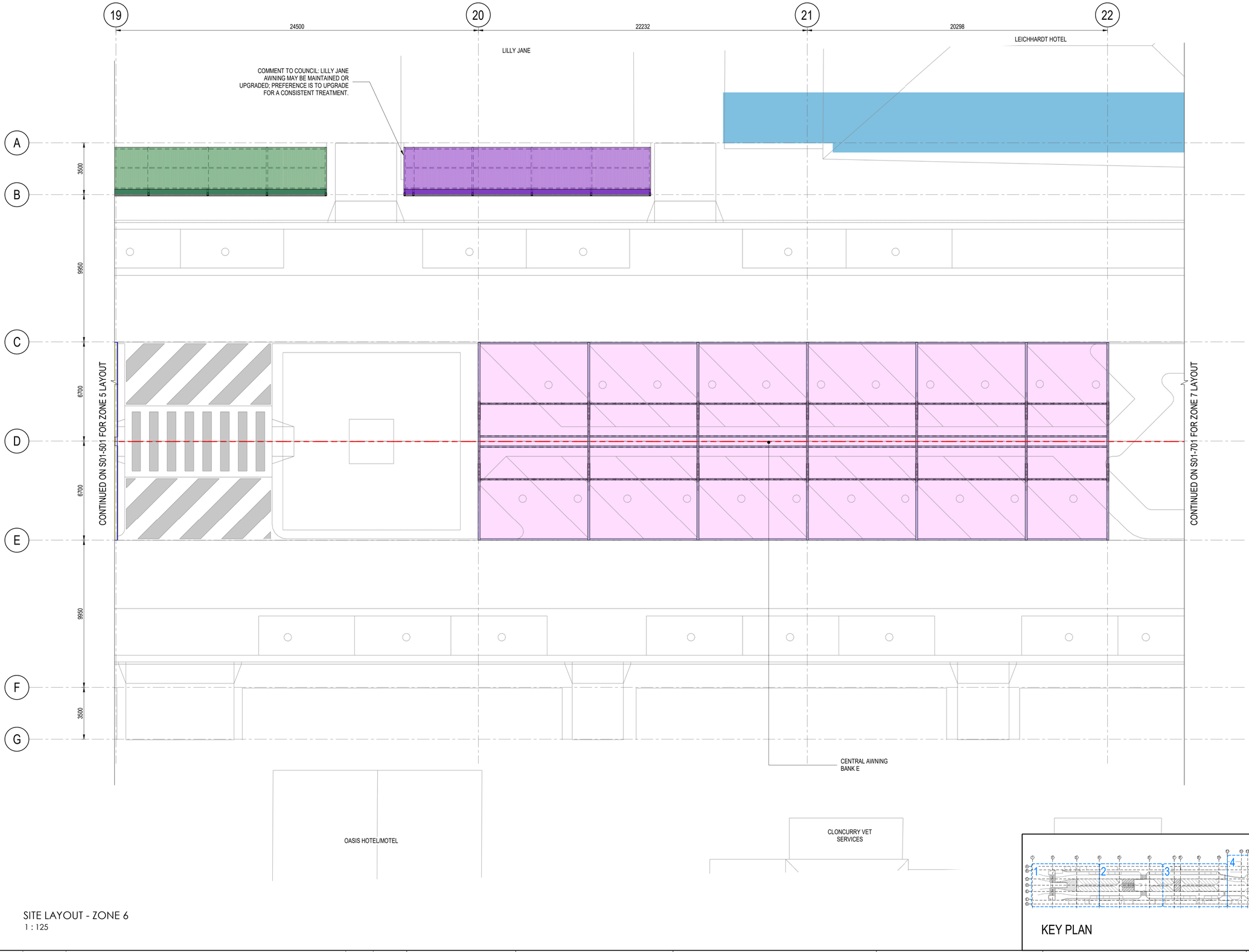

AWNING TREATMENT KEY

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- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
- PROPOSED NEW FOOTPATH AWNING
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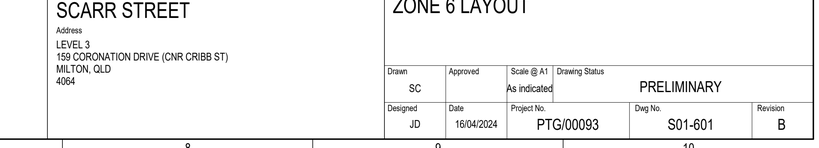
AWNING TREATMENT KEY

SCALE 1 : 1

SITE LAYOUT - ZONE 6

1 : 125



KEY PLAN

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

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North

Project

SCARR STREET

Address

LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title

ZONE 6 LAYOUT

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-601	B

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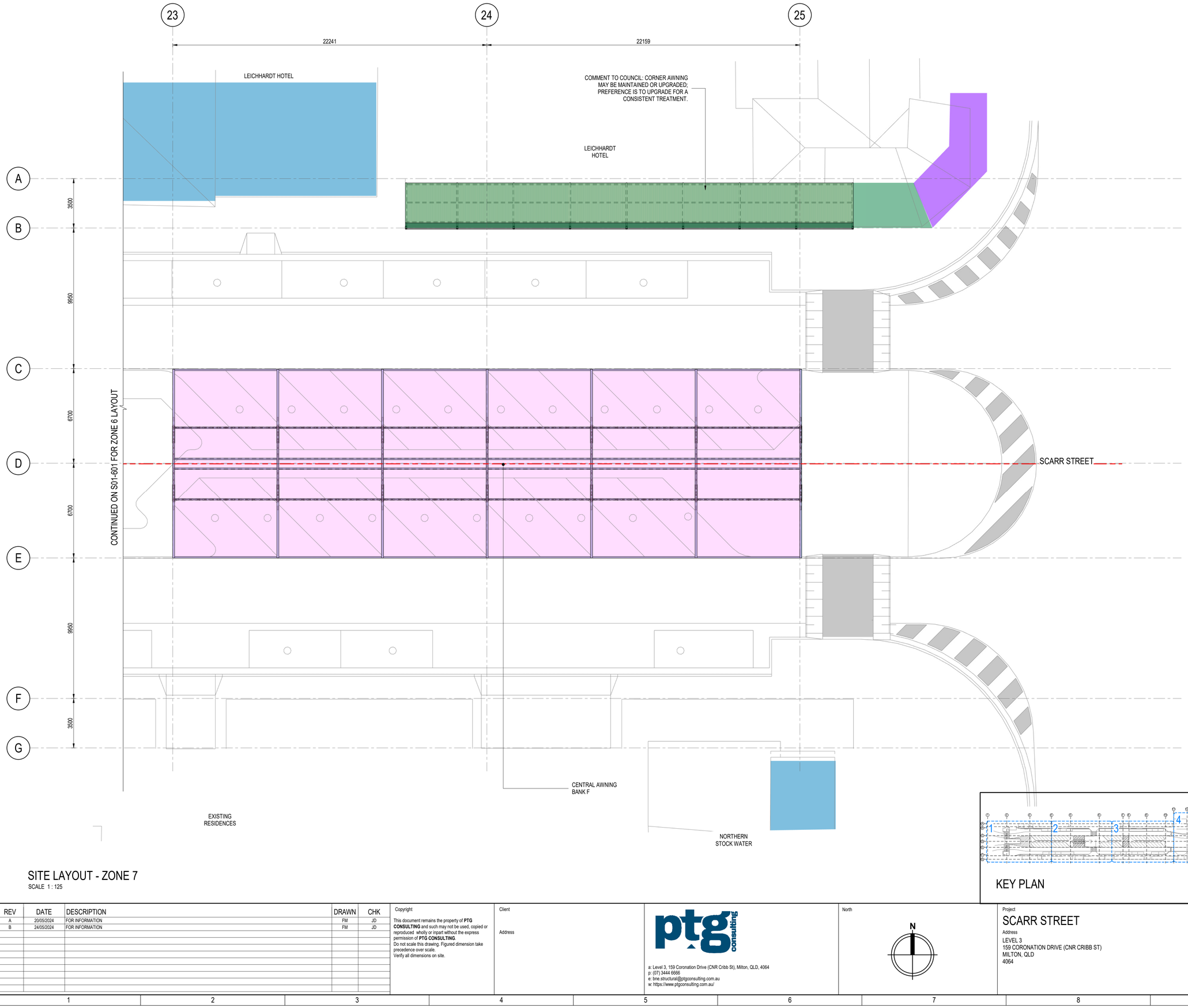

AWNING TREATMENT KEY

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- EXISTING AWNING TO BE DEMOLISHED
- PROPOSED NEW CENTRAL AWNING
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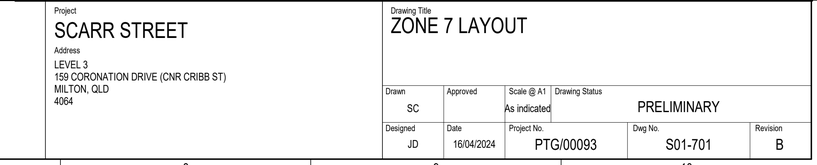

KEY PLAN

SITE LAYOUT - ZONE 7

SCALE 1 : 125

REV	DATE	DESCRIPTION	DRAWN	CHK
A	20/05/2024	FOR INFORMATION	FM	JD
B	24/05/2024	FOR INFORMATION	FM	JD

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Address

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Address
LEVEL 3
159 CORONATION DRIVE (CNR CRIBB ST)
MILTON, QLD
4064

Drawing Title

ZONE 7 LAYOUT

Drawn	Approved	Scale @ A1	Drawing Status
SC		As indicated	PRELIMINARY

Designed	Date	Project No.	Dwg No.	Revision
JD	16/04/2024	PTG/00093	S01-701	B

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